January 29, 2021

Carlos M Herdocia, PE Project Manager Stantec Inc. 901 Ponce de Leon Boulevard, Suite 900 Coral Gables FL 33134-3070

Subject:

Report of a Geotechnical Exploration – Roadway Soils Survey

Franjo Road, from Old Cutler Road to SW 184th Street

Town of Cutler Bay

Department of Public Works Miami-Dade County, Florida HRES Project No. HR19-1573R

#### Dear Carlos:

HR Engineering Services, Inc. (HRES) is pleased to provide this Report of a Geotechnical Exploration - Roadway Soils Survey for the subject project. This report presents our understanding of the project, outlines our exploratory procedures, and documents the field test data.

We have enjoyed assisting you on this project and look forward to serving as your geotechnical consultant on the remainder of this project and on future projects. If you have any questions concerning this report, please call our office at (305) 888-8880.

Sincerely,

HR ENGINEERING SERVICES, INC.

Chollada Soonyakanit, E.I.

MAT Staff Geotechnical Engineer

(Certificate of Authorization No. 7991)

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY

Hernando R Ramos

2021.02.09 14:05:11 -05'00'

ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED

ON ANY ELECTRONIC COPIES

Hernando R. Ramos, P.E.

MAT Chief Geotechnical Engineer Florida Registration 42045

Distribution:

Addressee (1) File (1)

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#### 1.0 INTRODUCTION

The purpose of this geotechnical exploration was to obtain information concerning the site and subsurface conditions along the proposed project. The project consists of the construction of the roadway widening of Franjo Road. This report discusses our exploratory and testing procedures, presents our findings and includes the following items:

#### **Field Services - HRES**

- Performed two (2) auger borings for the proposed roadway widening, each to a depth of 10.0 feet, measured from the existing ground surface. The borings subsurface information is presented on the Report of Core Borings and Soils Information Table in Appendix A.
- Forty-two (42) SPT borings performed along the proposed roadway widening, each to a depth of 10 feet. The test borings subsurface information is presented on the Report of Core Borings and Soils Information Table in Appendix A.
- Performed a total of nine (9) SFWMD usual open-hole constant head percolation tests. The tests were conducted at one depth interval, from 0 to 15 feet. The percolation test results are presented in Appendix A. The subsurface information is presented on the Soils Information Table in Appendix A.
- A brief description of our field testing procedures.

#### **Evaluation**

- Review of Miami-Dade County USDA Soil Survey Maps.
- Review of Miami-Dade County USGS Water Level Maps.
- Review of USGS Quadrangle Map.
- A general review of existing surface features and site conditions.
- Report of Core Borings and Soils Information Table.
- Roadway Soils Survey.
- Roadway construction recommendations.

#### **Laboratory Testing**

- The results of laboratory tests performed on selected soil samples obtained from the test borings.
- A brief description of our laboratory testing procedures.

#### 2.0 PROJECT INFORMATION

#### 2.1 GENERAL

Project information for this subsurface exploration has been provided to us by various members of the design team. Additional information has been provided during telephone conversations.

During our geotechnical study, we have been furnished with the following project-related plans and information:

Plans: Franjo Road, from Old Cutler Road to SW184 Street
 — Map of Topographic Survey Prepared by: N/A
 Dated: 11/03/2020

#### 2.2 PROJECT DESCRIPTION

The project consists of the roadway widening along Franjo Road, from Old Cutler Road to SW 184th Street. The project is located in Miami-Dade County Florida. This report provides recommendations for the construction of the roadway widening.

#### 3.0 FIELD EXPLORATION AND LABORATORY TESTING

#### 3.1 FIELD EXPLORATION

The field exploration was conducted by HRES. The locations of the test borings are provided in the Summary of Test Boring Locations in Appendix A. The Report of Core Borings and the Soils Information Table in Appendix A summarize the approximate boundary between soil types. In some instances, the transition between material types may be gradual. A brief description of the exploratory sampling techniques used is presented in the Field Testing Procedures section in Appendix A. A discussion of the subsurface conditions encountered along the project alignment is provided in Section 4.2 of this report.

#### 3.2 PERCOLATION TESTING

HRES completed nine (9) South Florida Water Management District (SFWMD) usual open-hole constant head percolation tests for the subject project. The percolation tests were conducted at one depth interval: from 0 to 15 feet. The hydraulic conductivity values were then calculated and reported in units of cubic feet per second, per square foot, per foot of head (cfs/ft²-ft of head). The calculated hydraulic conductivity values ranged as follows:

• From 0 to 15 ft.: 1.4E-04 cfs/ft<sup>2</sup>-ft to 8.2E-04 cfs/ft<sup>2</sup>-ft of head

The conductivity values are ultimate values. An appropriate factor of safety should be employed in any storm water or other subsurface drainage design computations. The percolation test results are presented in Appendix A.

#### 3.3 LABORATORY TESTING

#### 3.3.1 Soil Testing

In order to aid in classifying and estimate engineering characteristics of the subsurface materials encountered, laboratory classification tests were performed on representative soil samples obtained from the test borings. The laboratory testing program included the following:

- 6 Sieve Analyses
- 17 Fines Content Tests
- 2 Organic Content Tests
- 24 Moisture Content Tests

The soil laboratory test results were classified following the AASHTO Classification System. The test results are presented in Appendix B.

#### 4.0 SITE AND SUBSURFACE CONDITIONS

#### 4.1 SITE CONDITIONS

The site conditions were observed by a Geotechnical Engineer during the months of November and December, 2020.

#### **4.2 SUBSURFACE CONDITIONS**

#### 4.2.1 Miami-Dade County Soil Survey Map

The Soil Map of Miami-Dade County Area, Florida, published by the United States Department of Agriculture (USDA) was reviewed for general near-surface soil information within the general project vicinity. This information indicates that there is four (4) mapping unit in the vicinity of the project. The map soil unit encountered is:

**Table 4.2.1 Miami-Dade Soil Survey** 

Map Unit Symbol	Map Unit Name	Typical Profile
7	Krome very gravelly loam (4.7% of AOI)	Ap - 0 to 7 inches: very gravelly loam R - 7 to 11 inches: unweathered bedrock
10	Udorthents, limestone (65.5% of AOI)	C - 0 to 55 inches: extremely gravelly loam 2R - 55 to 59 inches: unweathered bedrock
11	Udorthents, marl substratum- Urban land complex (29.5% of AOI)	C1 - 0 to 12 inches: very gravelly loam C2 - 12 to 41 inches: gravelly sandy loam Cg - 41 to 80 inches: marly silt loam 2R - 80 to 90 inches: unweathered bedrock
99	Water (0.4% of AOI)	Water: 100 percent

A reproduction of the USDA map for the project area is included in Appendix A.

#### 4.2.2 USGS Quadrangle Map

The Perrine Quadrangle, Florida-Miami-Dade Topographic Map (1999) published by the United States Geological Survey (USGS) was reviewed for general existing ground surface elevations in

January 29, 2021 Project No. HR19-1573R

the project area. Based on the map, the existing ground elevations in the project vicinity is 10.0 feet, NGVD29. A reproduction of the USGS Quadrangle Map for the project area is included in Appendix A.

#### 4.2.3 Generalized Subsurface Conditions Encountered Along the Alignment

A total of seven different layers of materials were observed during the performance of the boreholes. Stratum 1a is asphalt. Stratum 1b is topsoil. Stratum 2 consists of limerock with silty fine sand. Stratum 3 consists of silty fine sand with traces limerock fragments. Stratum 4 consists of sandy silt/slightly organic sandy silt with traces of limerock fragments. Stratum 5 consists of fine sand with traces limerock fragments. Stratum 6 consists of the natural limestone. For a detailed subsurface condition at a particular borehole location, please refer to the Report of Core Borings and the Soils Information Table in Appendix A.

#### 4.2.4 Groundwater Conditions

The groundwater levels in the borings were measured at the time of drilling. Groundwater levels were encountered at depths ranging from 4.0 to 10.8 feet.

The average October Water Level was 3.0 feet, NGVD29 (1.5 feet, NAVD88) and the Seasonal High Ground Water Table is about 4.0 feet, NGVD29 (2.5 feet, NAVD88) were found for the project area based on U.S. Geological Survey (2002), "Average Altitude of the Water Table (1990-99) and Frequency Analysis of Water Levels (1974-99) in Biscayne Aquifer, Miami-Dade County, Florida" included in Appendix A.

Fluctuation in the observed groundwater levels should be expected due to seasonal climatic changes, construction activity, rainfall variations, a storm surge, surface water runoff and water level variations in the nearby lakes. Since groundwater level variations are anticipated, design drawings and specifications should accommodate such possibilities and construction planning should be based on the assumption that variations will occur.

#### 5.0 ROADWAY CONSTRUCTION RECOMMENDATIONS

#### **5.1 BASIS FOR RECOMMENDATIONS**

The following construction recommendations are based upon our understanding of the conceptual design information available at the writing of this report and the data gathered during our subsurface exploration. The stratification of the subsurface materials underlying the site may vary within even short lateral distances; therefore, any subsurface condition encountered which differs from those documented in this study should be reported to us so that our recommendations can be reviewed.

#### 5.2 SUITABILITY OF IN-SITU MATERIALS

The following is a summary of the subsurface information provided by the borings and their suitability.

<u>Stratum 1a</u> – This Stratum consists of asphalt.

<u>Stratum 1b</u> – This Stratum consists of dark brown organic silty fine sand (topsoil, A-8). No laboratory testing was conducted on this material. This material is unsuitable for use as stabilized subgrade or fill material and should be removed.

<u>Stratum 2</u> – This Stratum consists of brown to light brown limerock fragments with silty fine sand (fill, A-1-b). Laboratory testing on this material consisted of 3 sieve analyses and 3 moisture content tests. The fines content ranged from 15 to 21 percent and the moisture content ranged from 11 to 18 percent. This material appears suitable for use as a general fill when utilized in accordance with FDOT Index 120-001. It cannot be used as base material.

Stratum 3 – This stratum consists of brown silty fine sand with traces of limerock fragments (A-2-4). Laboratory testing on this material consisted of 3 sieve analyses, 3 fines content tests and 6 moisture content tests. The fines content ranged from 23 to 35 percent and the moisture content ranged from 11 to 36 percent. This material appears suitable for use in the embankment when utilized in accordance with FDOT Standard Plan Index 120-001. However, this material is likely to retain excess moisture and be difficult to dry and compact. It should be used in the embankment above the water level existing at the time of construction. It cannot be used as stabilized subgrade or base material.

feet below the bottom of the base.

Stratum 4 – This Stratum consists of brown sandy silt or slightly organic sandy silt with traces of limerock fragments (A-4). Laboratory testing on this material consisted of 2 organic content tests, 13 fines content tests and 14 moisture content tests. The organic content ranged from 4 to 5, the fines content ranged from 50 to 98 percent and the moisture content ranged from 32 to 75 percent. This material is unsuitable for use in the embankment and as stabilized subgrade and shall be removed in accordance with FDOT Standard Plan Index 120-002. It should be removed if encountered within 2

This material was encountered within 2 feet below the bottom of the base at the following borehole locations: RB-28, RB-32, RB-36, RB-39, RB-40 and Percolations Tests P-7, P-8 and P-9.

<u>Stratum 5</u> – This Stratum consists of brown fine sand with traces of limerock fragments (A-3). Laboratory testing on this material consisted of 1 fines content test and 1 moisture content test. The fines content was 8 percent and the moisture content was 25 percent. This material appears suitable for use in the embankment when utilized in accordance with FDOT Standard Index 120-001. It cannot be used as stabilized subgrade or base material.

<u>Stratum 6</u> – This stratum consists of the natural limestone. This material appears suitable for use as general fill when utilized in accordance with FDOT Standard Plan Index 120-001. This material typically offers a high resistance to excavation. Special equipment and breaking tools may be required to excavate it. This material is also difficult to dewater due to its high porosity and permeability.

#### 5.3 CONSTRUCTION PLANS AND SPECIFICATIONS REVIEW

It is recommended that this office be provided the opportunity to make a general review of the earthwork plans and special provisions prepared from the recommendations presented in this report. We would then suggest any modifications so that our recommendations are properly interpreted and implemented.

#### 5.4 SETTLEMENT AND VIBRATION MONITORING

Construction vibrations associated compaction equipment may occur. Settlement and vibration monitoring should be performed in accordance with Section 108 of FDOT Standard Specifications.

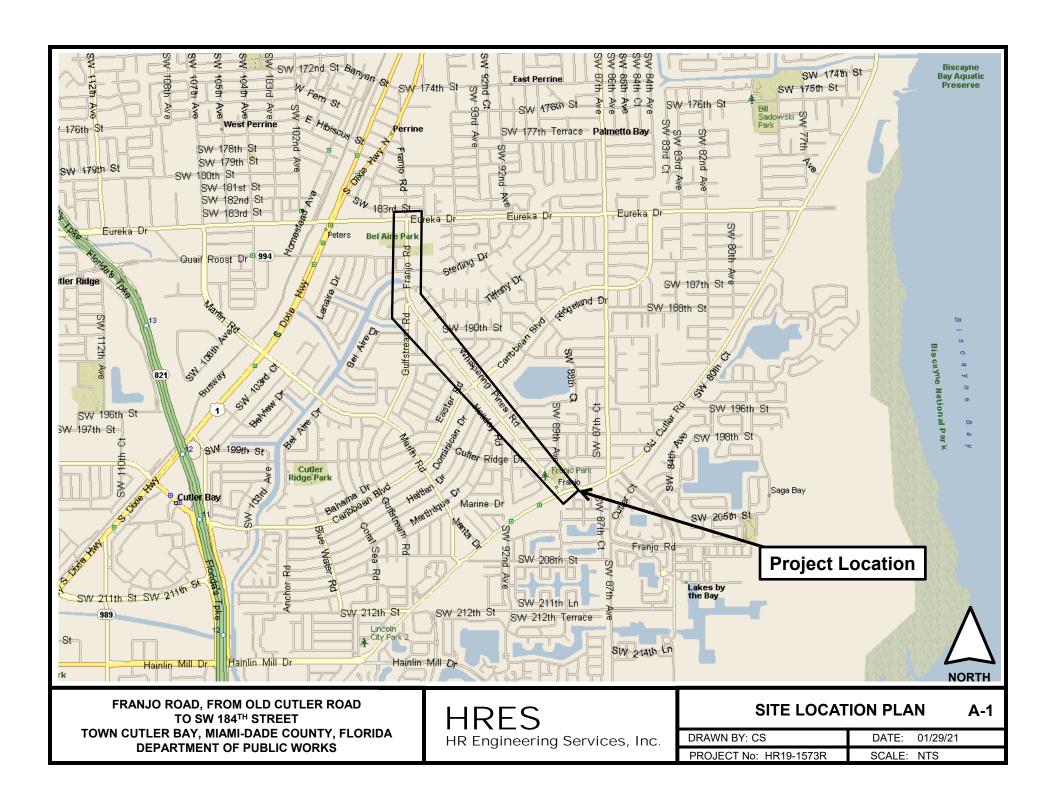
#### **6.0 REPORT LIMITATIONS**

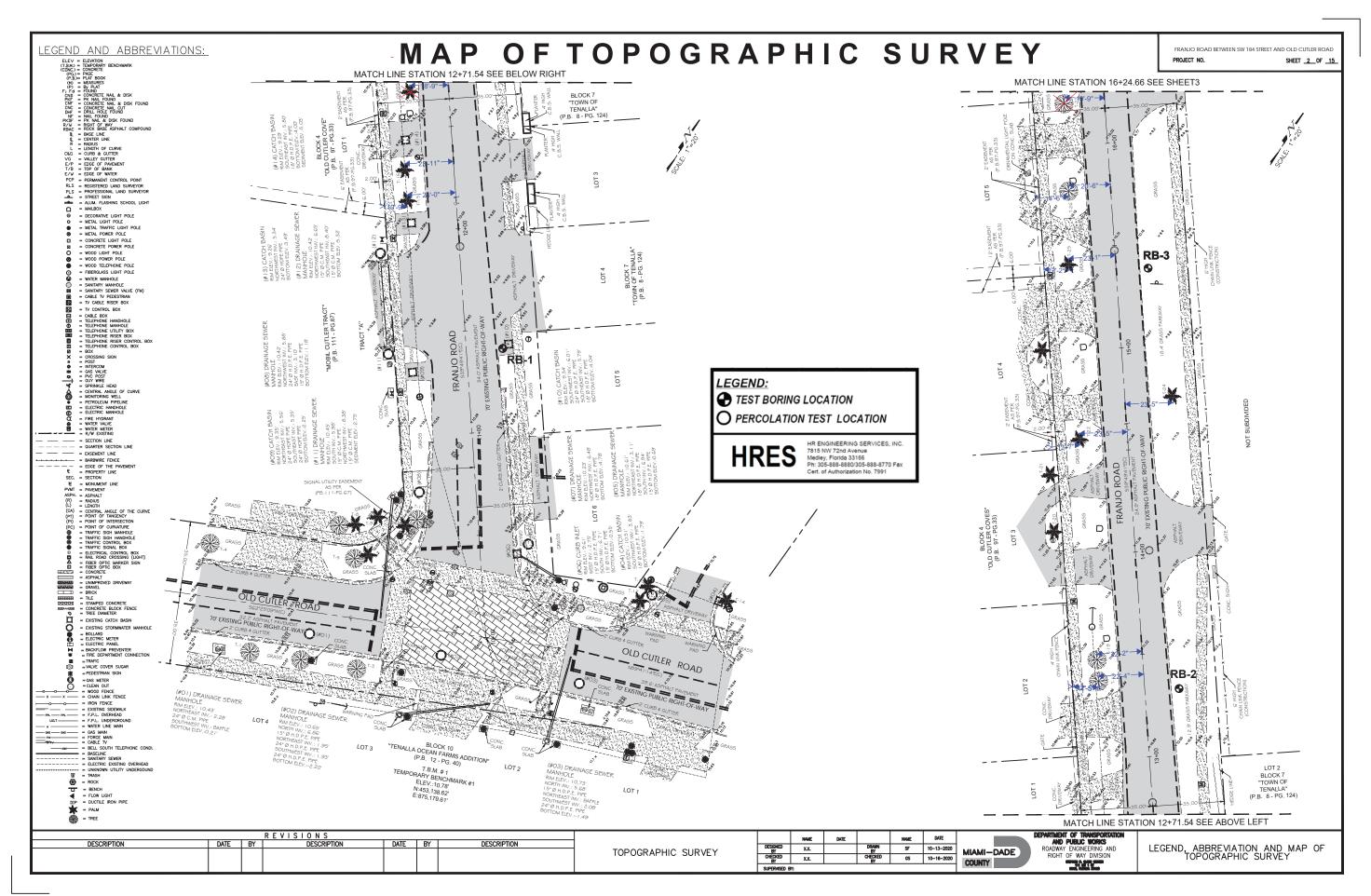
The scope of the investigation was intended to evaluate the subsurface conditions along the proposed roadway improvements. The analyses and recommendations submitted in this report are based upon the data obtained from the test borings performed at the locations indicated. The applicability of the report should also be reviewed in the event significant changes occur in the design, nature or location of the proposed improvement.

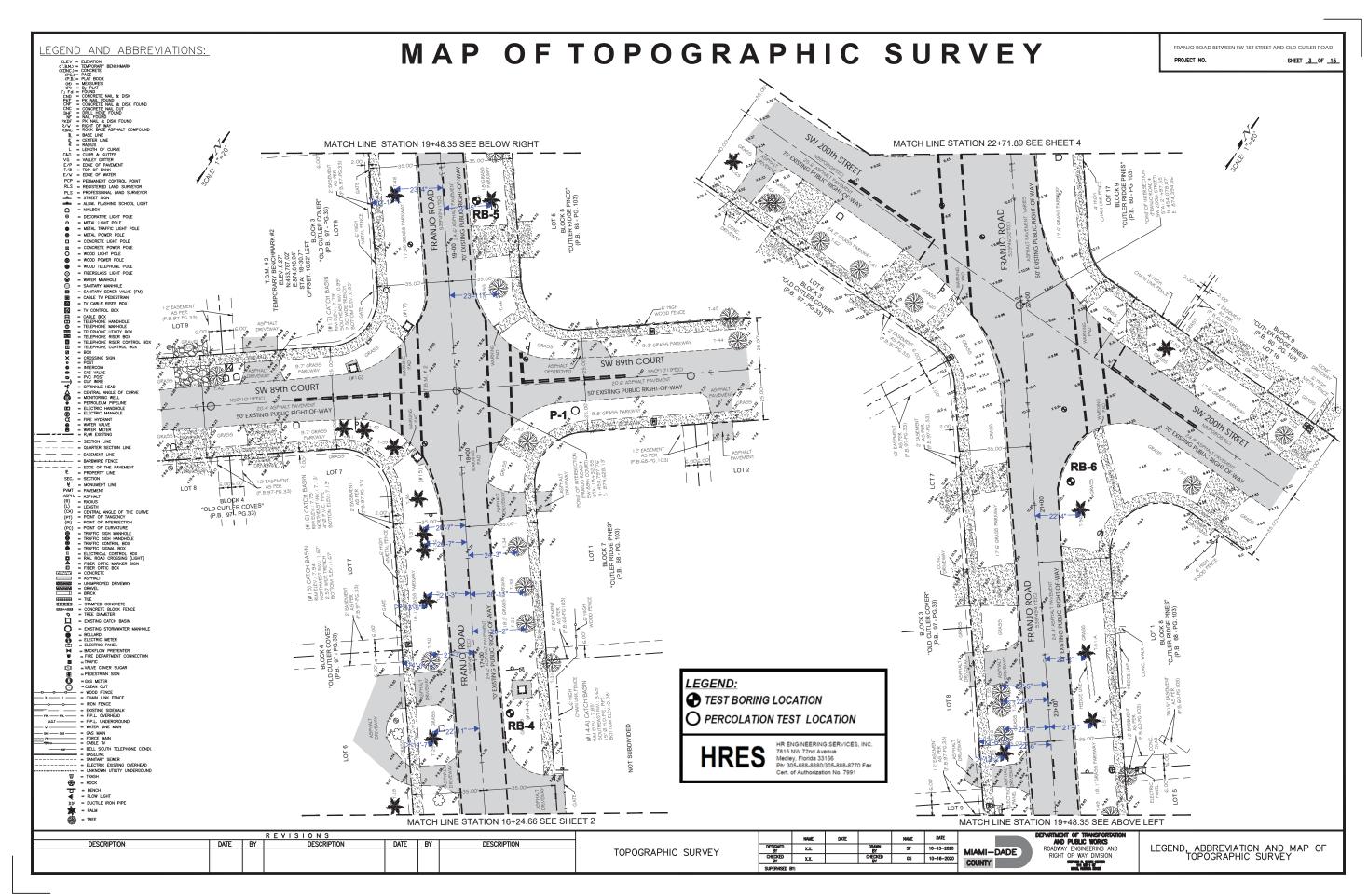
The scope of our services does not include any environmental assessment or investigation for the presence or absence of hazardous or toxic material in the soil, groundwater, or surface water within or beyond the site studied.

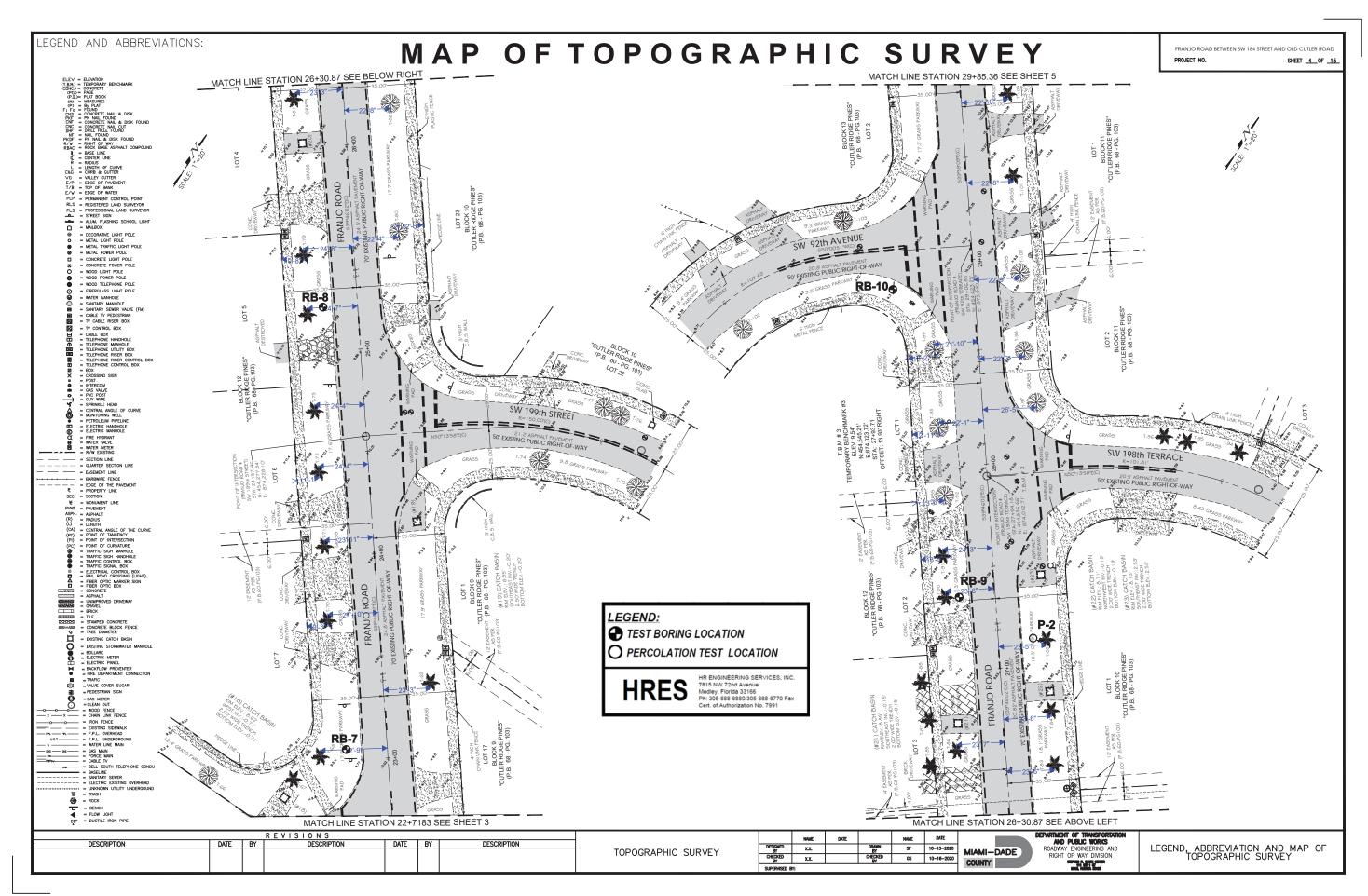
# **APPENDIX A**

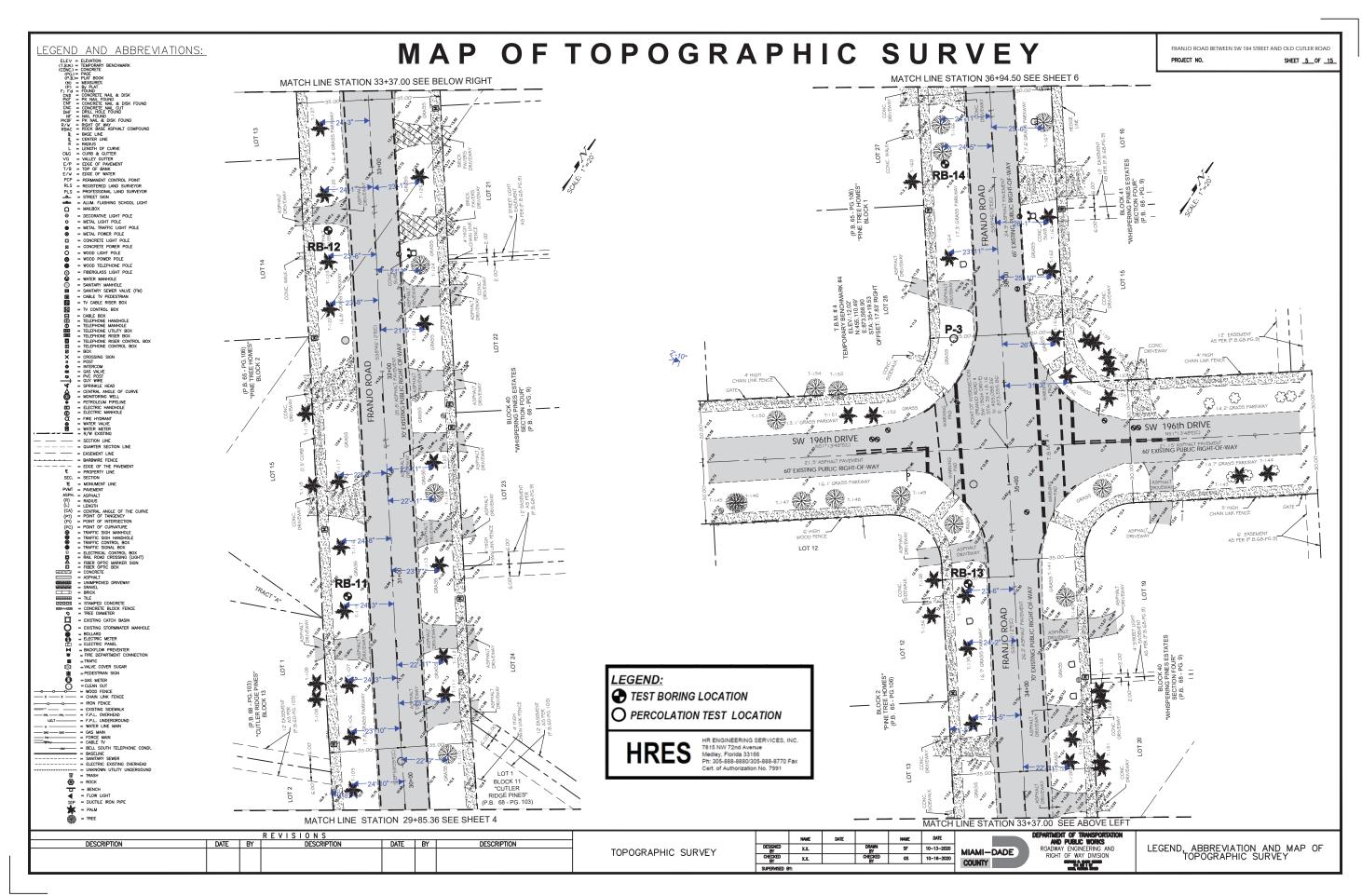
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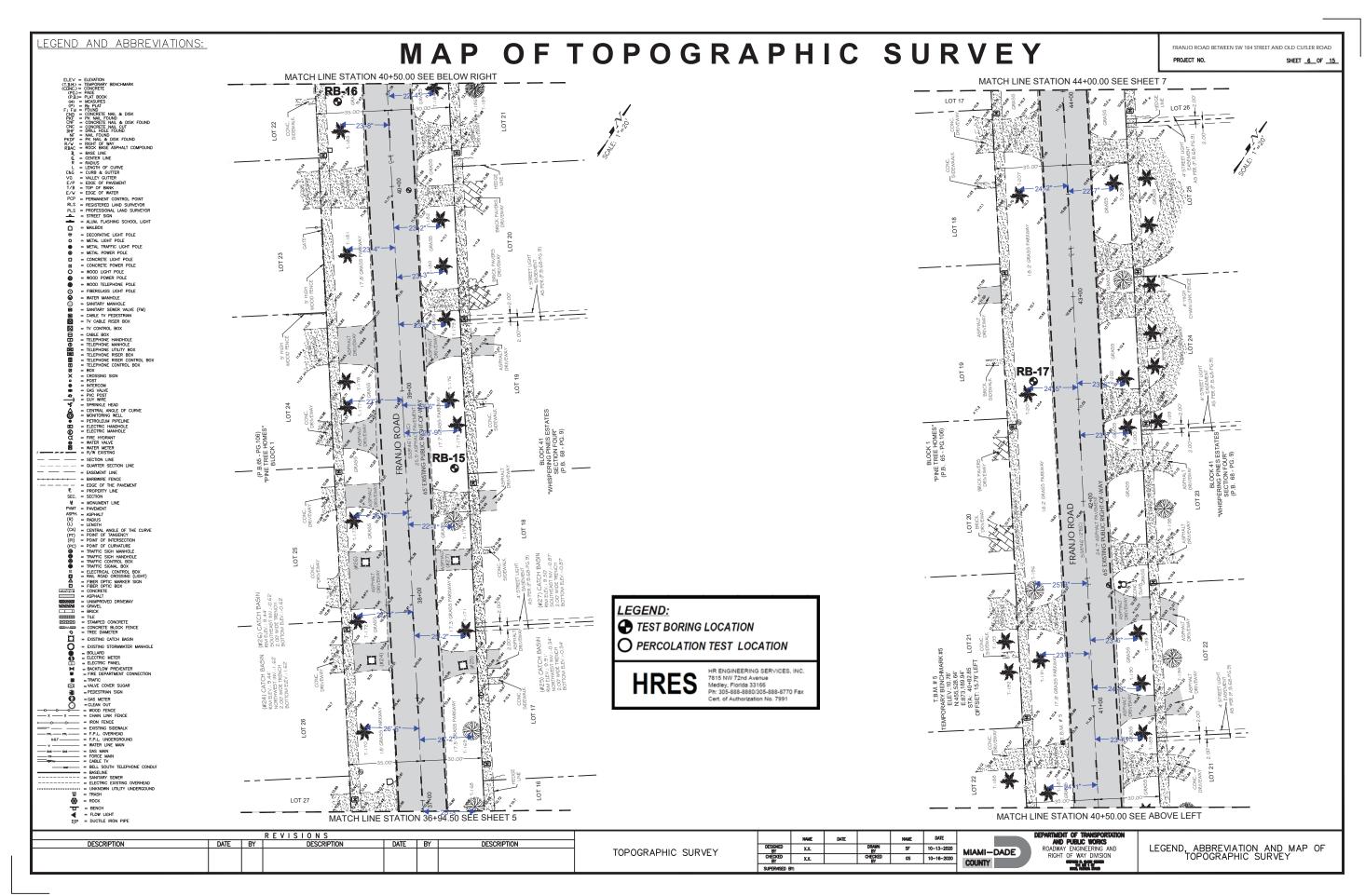


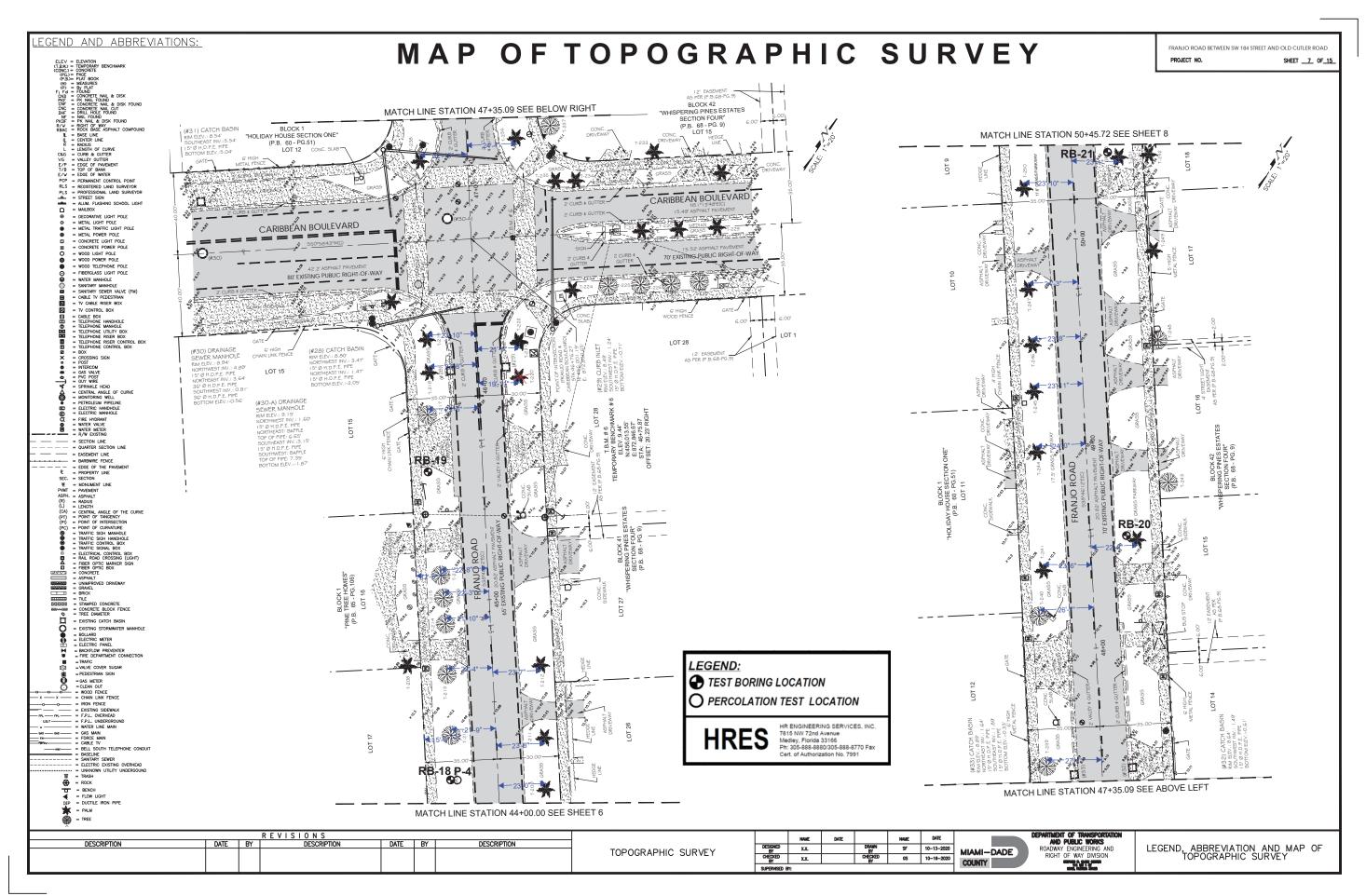


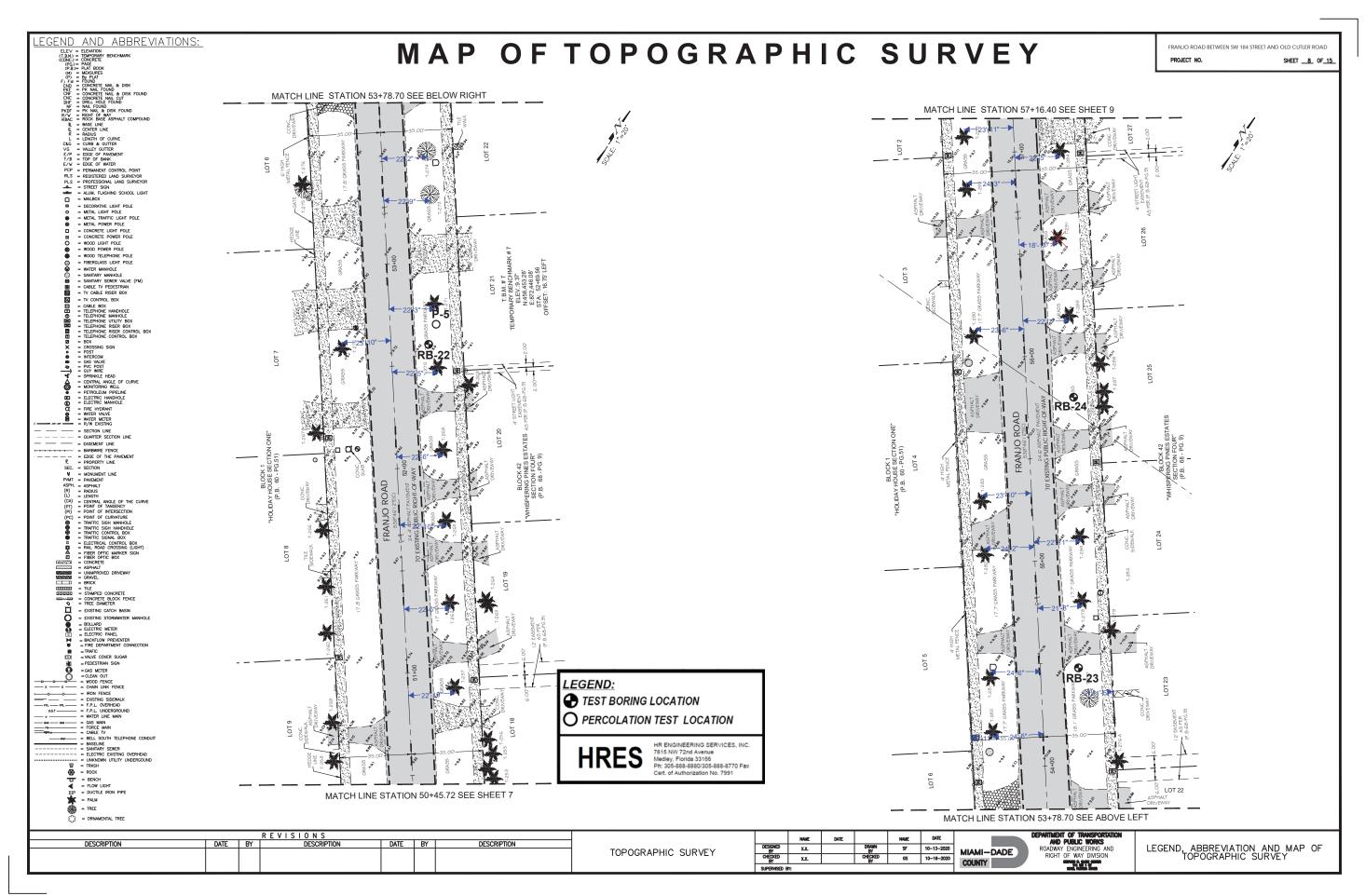


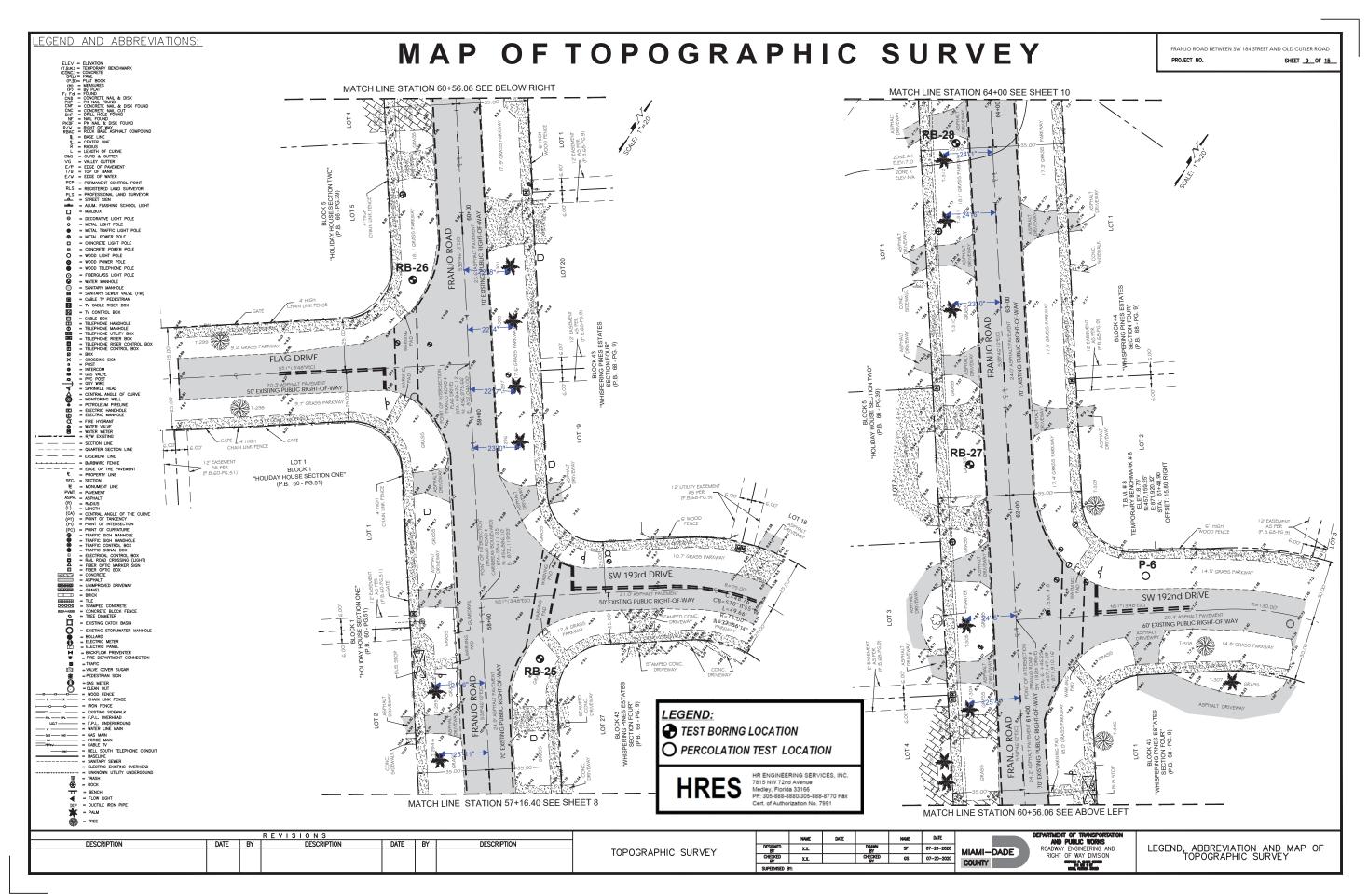


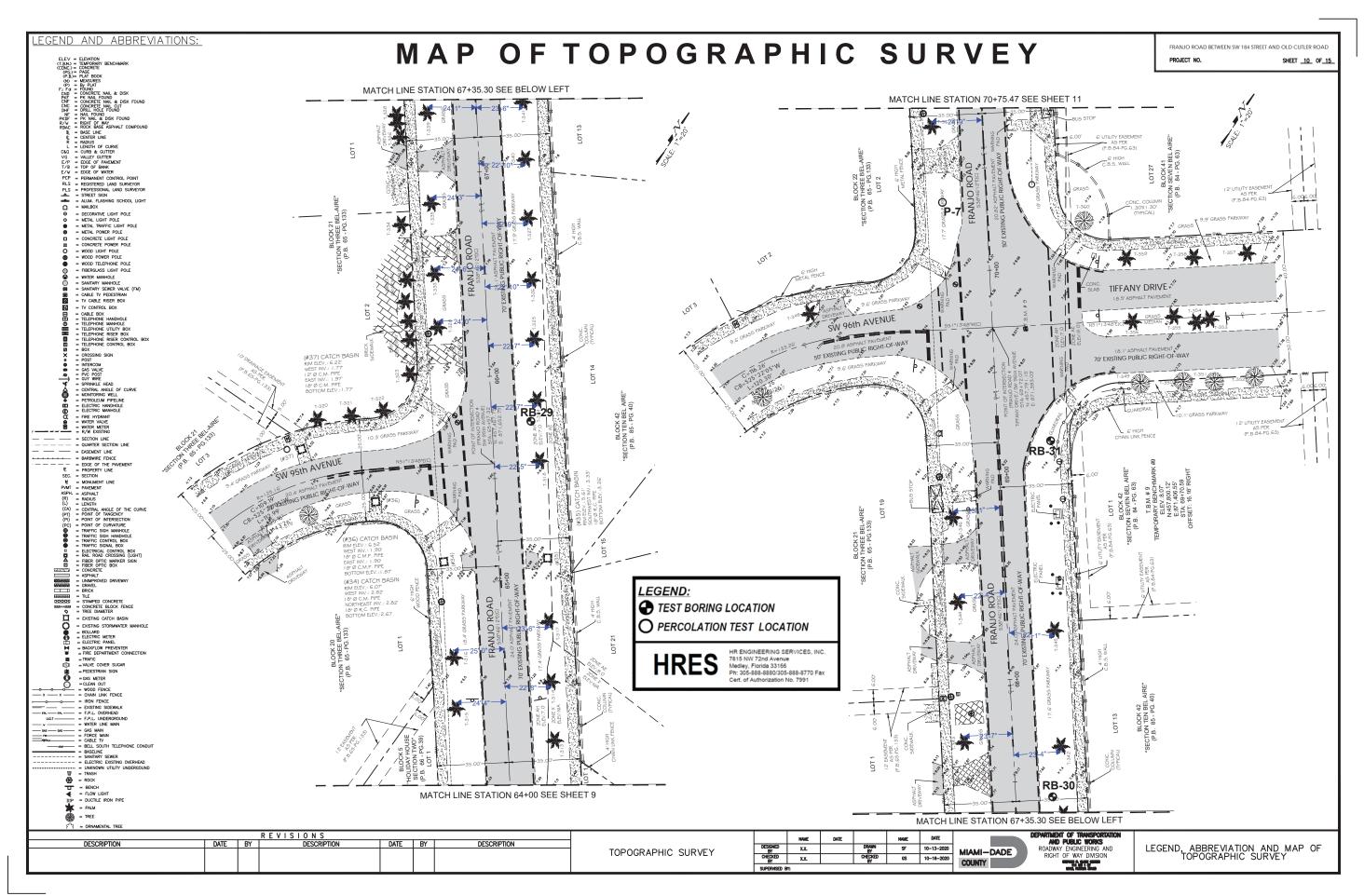


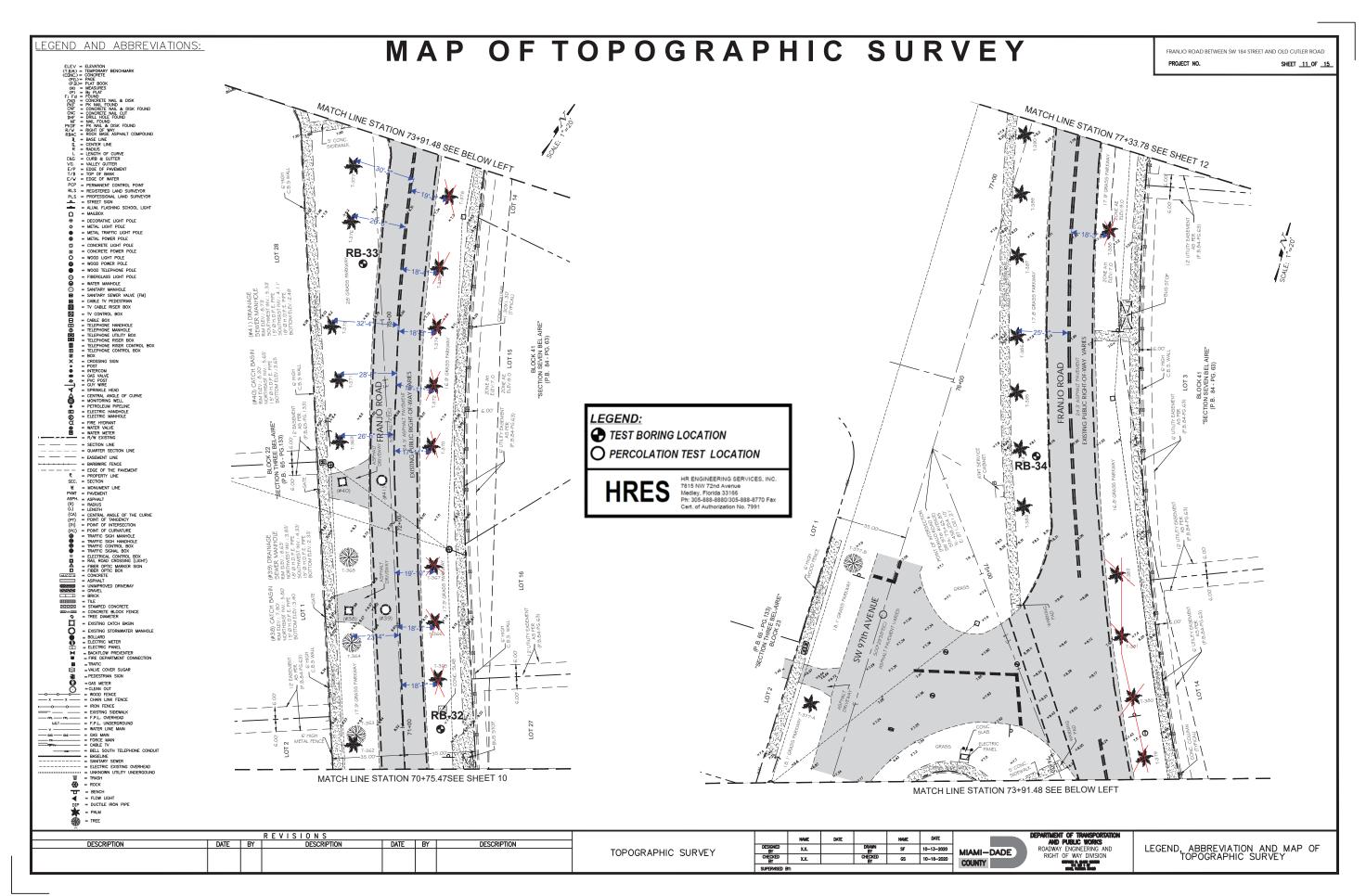


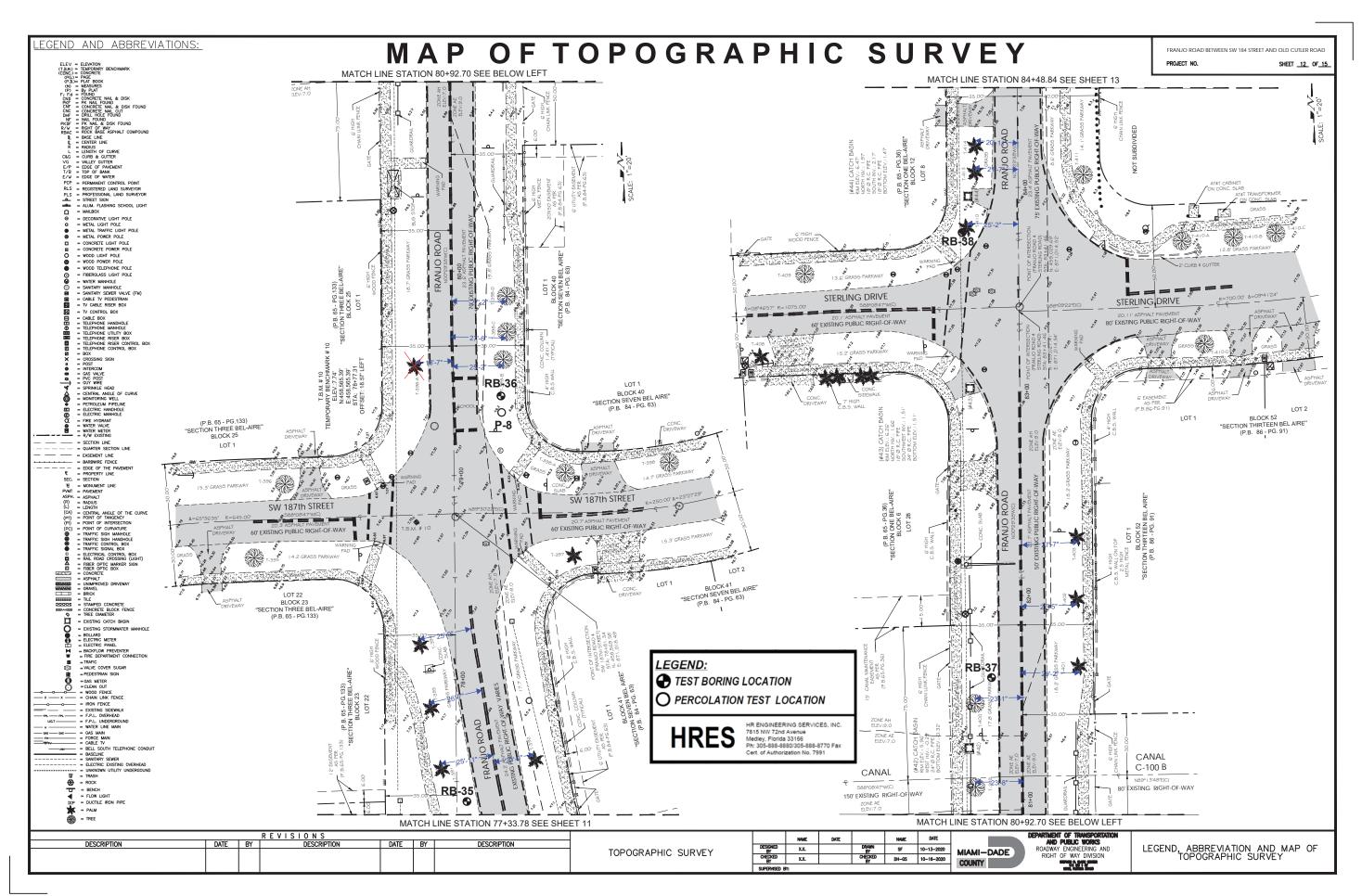


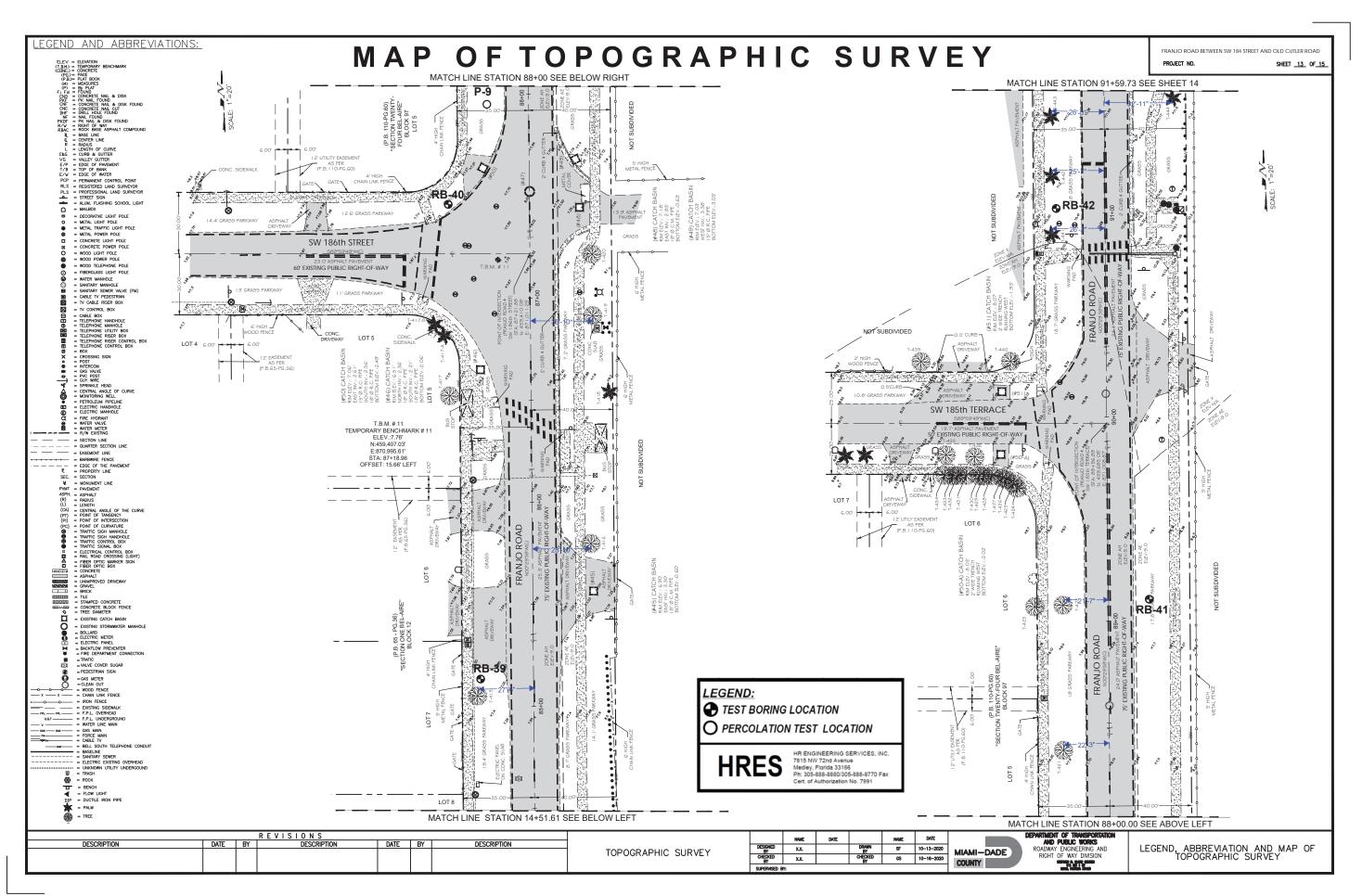


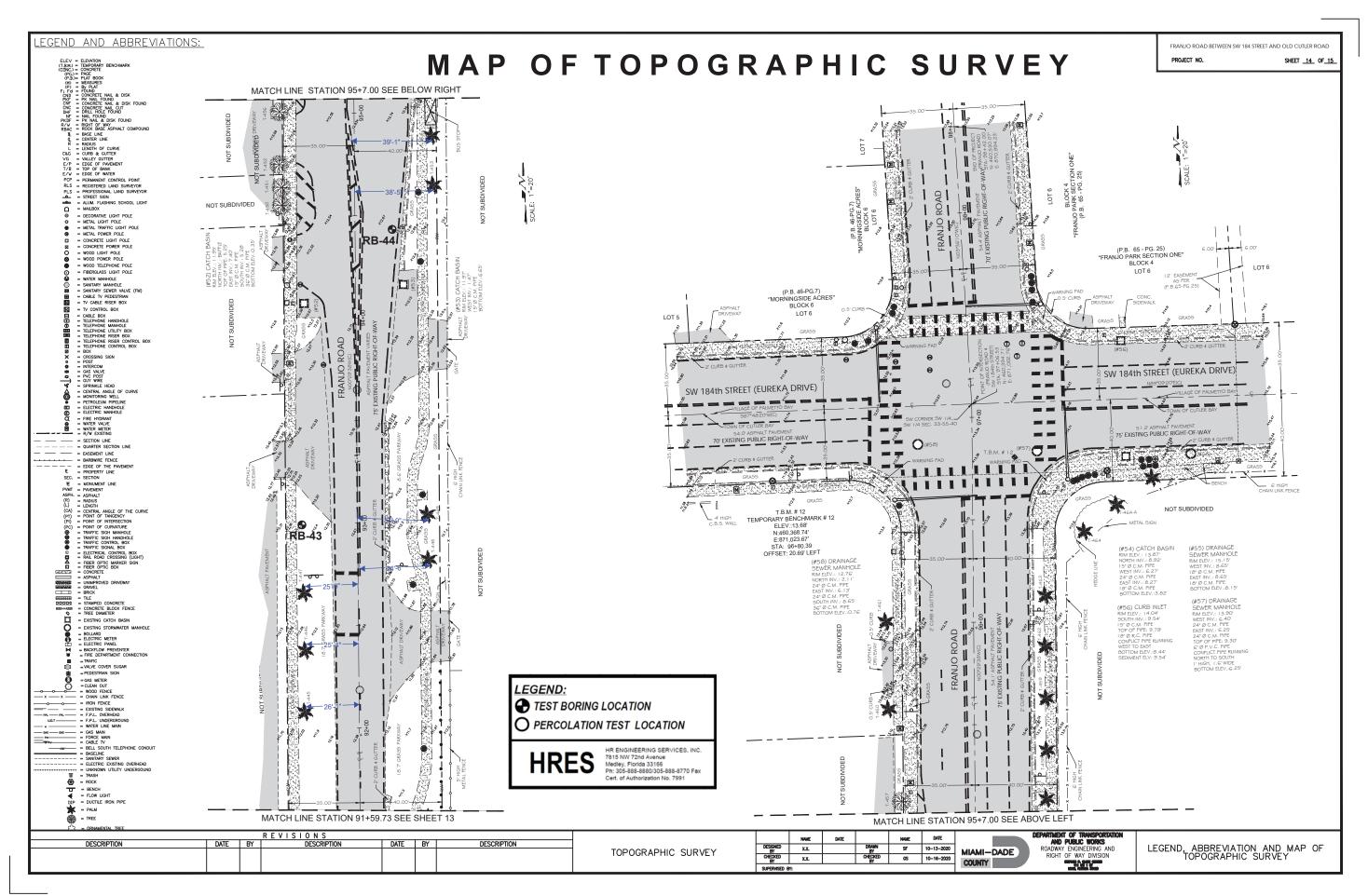


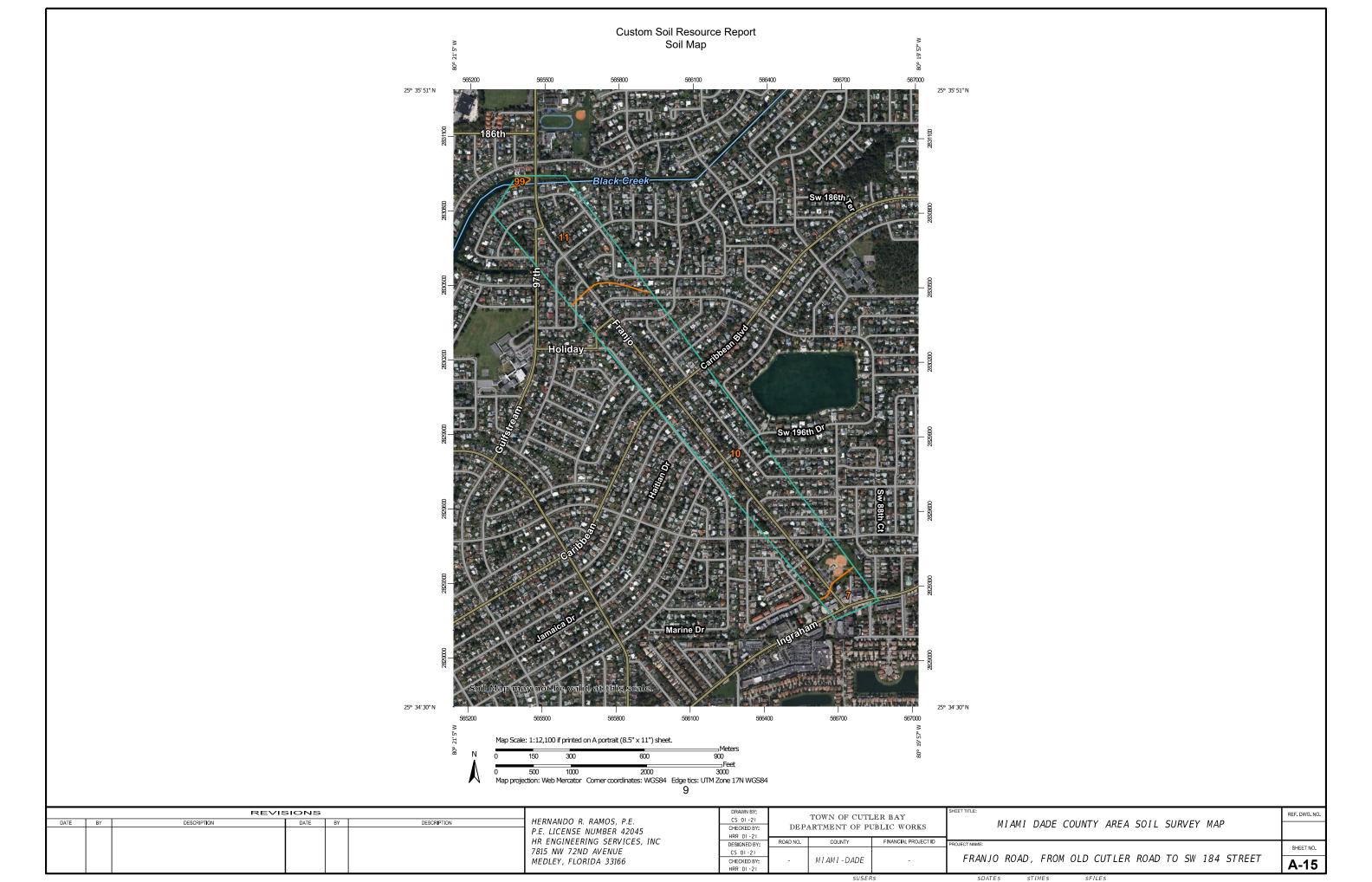


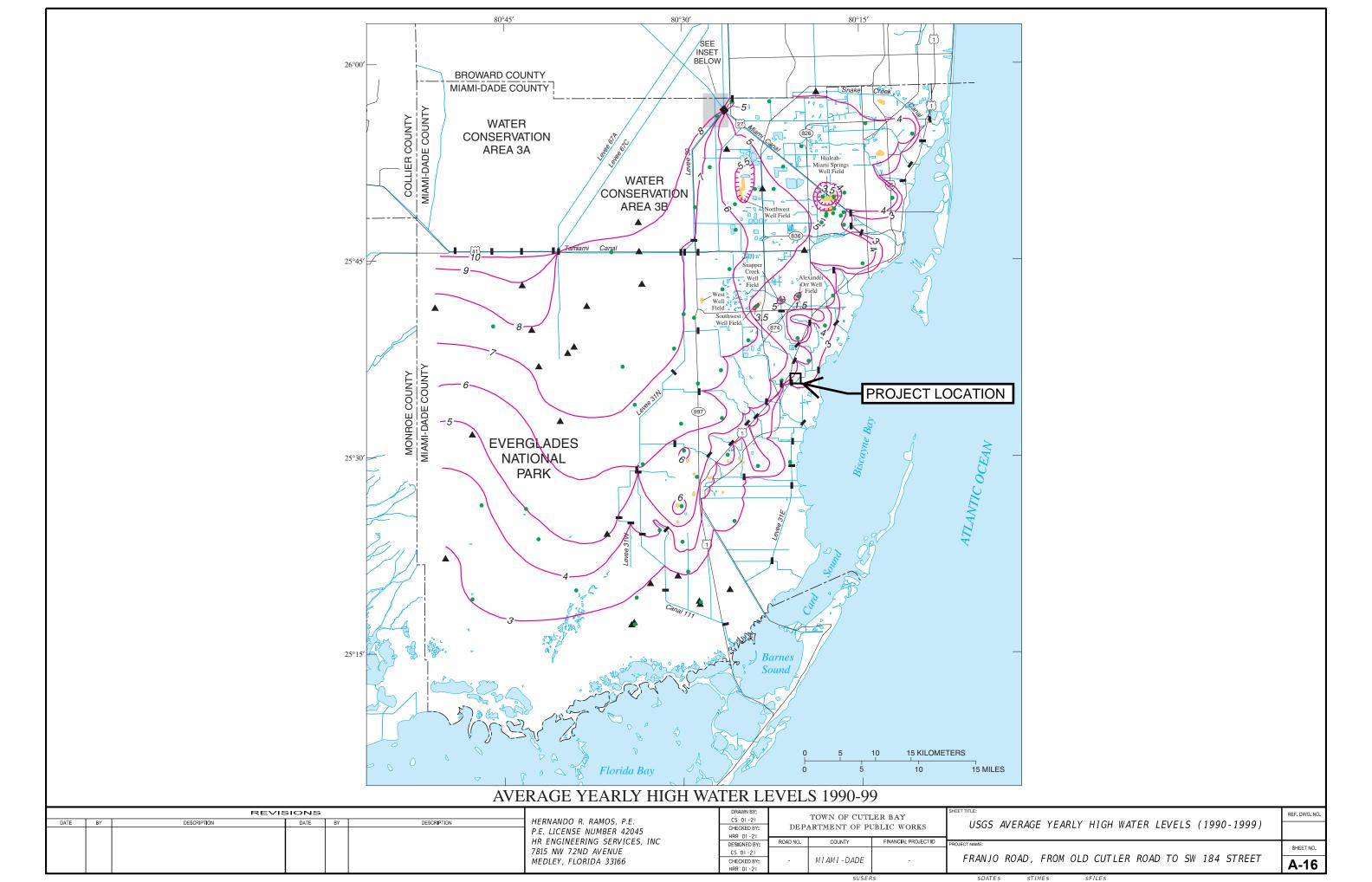


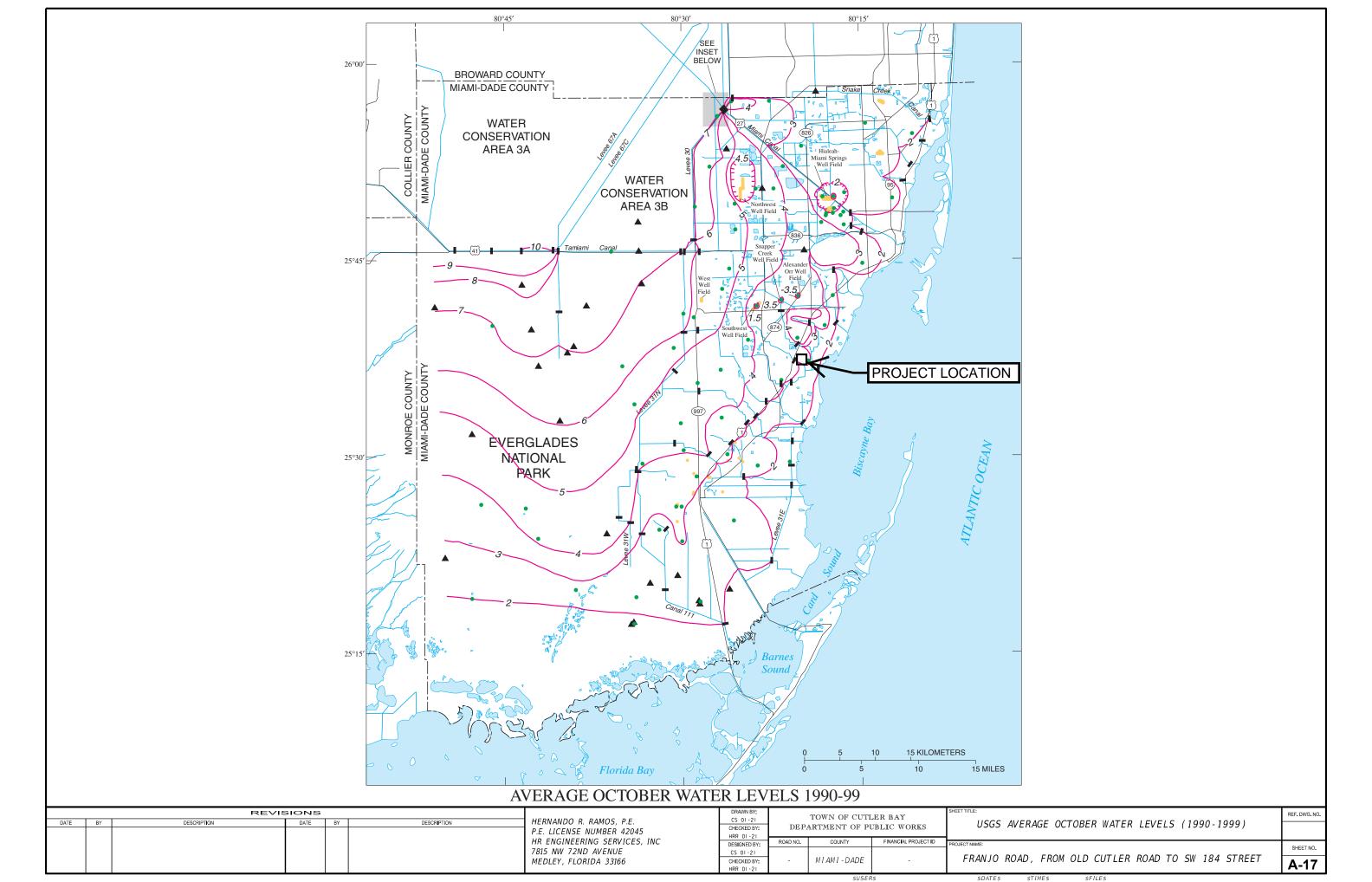


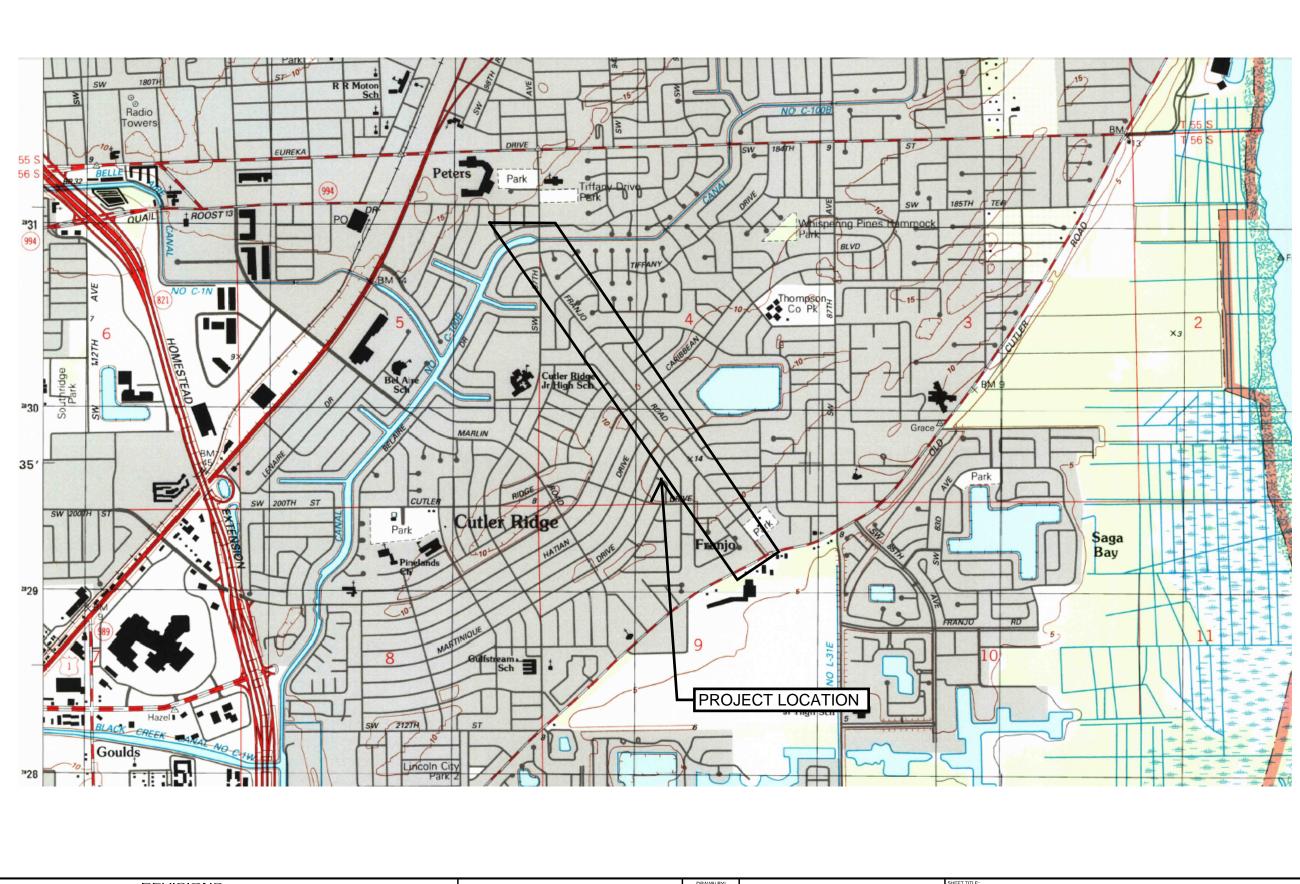












REVISIONS REF. DWG. NO. TOWN OF CUTLER BAY HERNANDO R. RAMOS, P.E. DESCRIPTION DESCRIPTION DATE USGS QUADRANGLE ELEVATION MAP DEPARTMENT OF PUBLIC WORKS CHECKED BY: P.E. LICENSE NUMBER 42045 HR ENGINEERING SERVICES, INC FINANCIAL PROJECT ID COUNTY DESIGNED BY: SHEET NO. 7815 NW 72ND AVENUE FRANJO ROAD, FROM OLD CUTLER ROAD TO SW 184 STREET MIAMI-DADE MEDLEY, FLORIDA 33166 CHECKED BY A-18

## SUMMARY OF BORINGS LOCATIONS FRANJO ROAD, FROM OLD CUTLER ROAD TO SW 184<sup>1H</sup> STREET TOWN OF CUTLER BAY

# MIAMI-DADE COUNTY, FLORIDA

### **DEPARTMENT OF PUBLIC WORKS**

HR ENGINEERING SERVICES, INC. HRES PROJECT No. HR19-1573R JANUARY 29, 2021

Boring No.	Plane co	ordinates	- Station	Offset
Doming No.	Latitude	Longitude	Otation	ft.
RB-1	25.57913	-80.33584	11+40	18.0 R
RB-2	25.57955	-80.33621	13+30	17.0 R
RB-3	25.57999	-80.33663	15+40	15.0 R
RB-4	25.58028	-80.33689	16+75	15.0 R
P-1	25.58065	-80.33706	18+17	58.0 R
RB-5	25.58080	-80.33736	19+24	16.0 R
RB-6	25.58120	-80.33771	21+10	18.0 R
RB-7	25.58155	-80.33818	23+05	21.0 L
RB-8	25.58202	-80.33860	25+22	18.0 L
P-2	25.58248	-80.33888	27+15	18.0 L
RB-9	25.58246	-80.33900	27+35	18.0 L
RB-10	25.58276	-80.33935	28+88	43.0 L
RB-11	25.58321	-80.33970	30+90	22.0 L
RB-12	25.58361	-80.34004	32+70	22.0 L
RB-13	25.58400	-80.34038	34+53	22.0 L
P-3	25.58425	-80.34061	35+71	23.0 L
RB-14	25.58443	-80.34077	36+56	24.0 L
RB-15	25.58495	-80.34104	38+62	23.0 R
RB-16	25.58527	-80.34150	40+42	24.0 L
RB-17	25.58573	-80.34191	42+59	23.0 L
RB-18	25.58606	-80.34220	44+13	26.0 L
P-4	25.58607	-80.34221	44+16	26.0 L
RB-19	25.58638	-80.34248	45+61	26.0 L
RB-20	25.58708	-80.34293	48+54	18.0 R
RB-21	25.58748	-80.34329	50+41	17.0 R
RB-22	25.58795	-80.34369	52+58	18.0 R
P-5	25.58797	-80.34371	52+68	18.0 R
RB-23	25.58836	-80.34405	54+59	19.0 R

## SUMMARY OF BORINGS LOCATIONS FRANJO ROAD, FROM OLD CUTLER ROAD TO SW 184<sup>1H</sup> STREET TOWN OF CUTLER BAY

# MIAMI-DADE COUNTY, FLORIDA

## **DEPARTMENT OF PUBLIC WORKS**

HR ENGINEERING SERVICES, INC. HRES PROJECT No. HR19-1573R JANUARY 29, 2021

Doving No.	Plane co	ordinates	Station	Offset
Boring No.	Latitude	Longitude	- Station	ft.
RB-24	25.58865	-80.34430	55+79	23.0 R
RB-25	25.58909	-80.34466	57+80	28.0 R
RB-26	25.58940	-80.34514	59+68	25.0 L
P-6	25.58998	-80.34531	61+65	64.0 R
RB-27	25.58996	-80.34560	62+21	18.0 L
RB-28	25.59031	-80.34589	63+78	17.0 L
RB-29	25.59079	-80.34619	65+77	19.0 R
RB-30	25.59115	-80.34651	67+42	19.0 R
RB-31	25.59154	-80.34682	69+14	24.0 R
P-7	25.59171	-80.34715	70+34	23.0 L
RB-32	25.59191	-80.34717	70+99	18.0 R
RB-33	25.59236	-80.34766	73+27	8.0 L
RB-34	25.59292	-80.34795	75+80	50.0 R
RB-35	25.59337	-80.34808	77+40	4.0 R
P-8	25.59390	-80.34804	79+30	23.0 R
RB-36	25.59392	-80.34804	79+38	23.0 R
RB-37	25.59453	-80.34815	81+59	15.0 L
RB-38	25.59591	-80.34819	83+77	28.0 L
RB-39	25.59556	-80.34818	85+12	26.0 L
RB-40	25.59616	-80.34822	87+45	38.0 L
P-9	25.59627	-80.34817	87+95	20.0 L
RB-41	25.59659	-80.34805	89+10	20.0 R
RB-42	25.59712	-80.34818	91+02	23.0 L
RB-43	25.59766	-80.34820	93+00	25.0 L
RB-44	25.59805	-80.34807	94+42	19.0 R

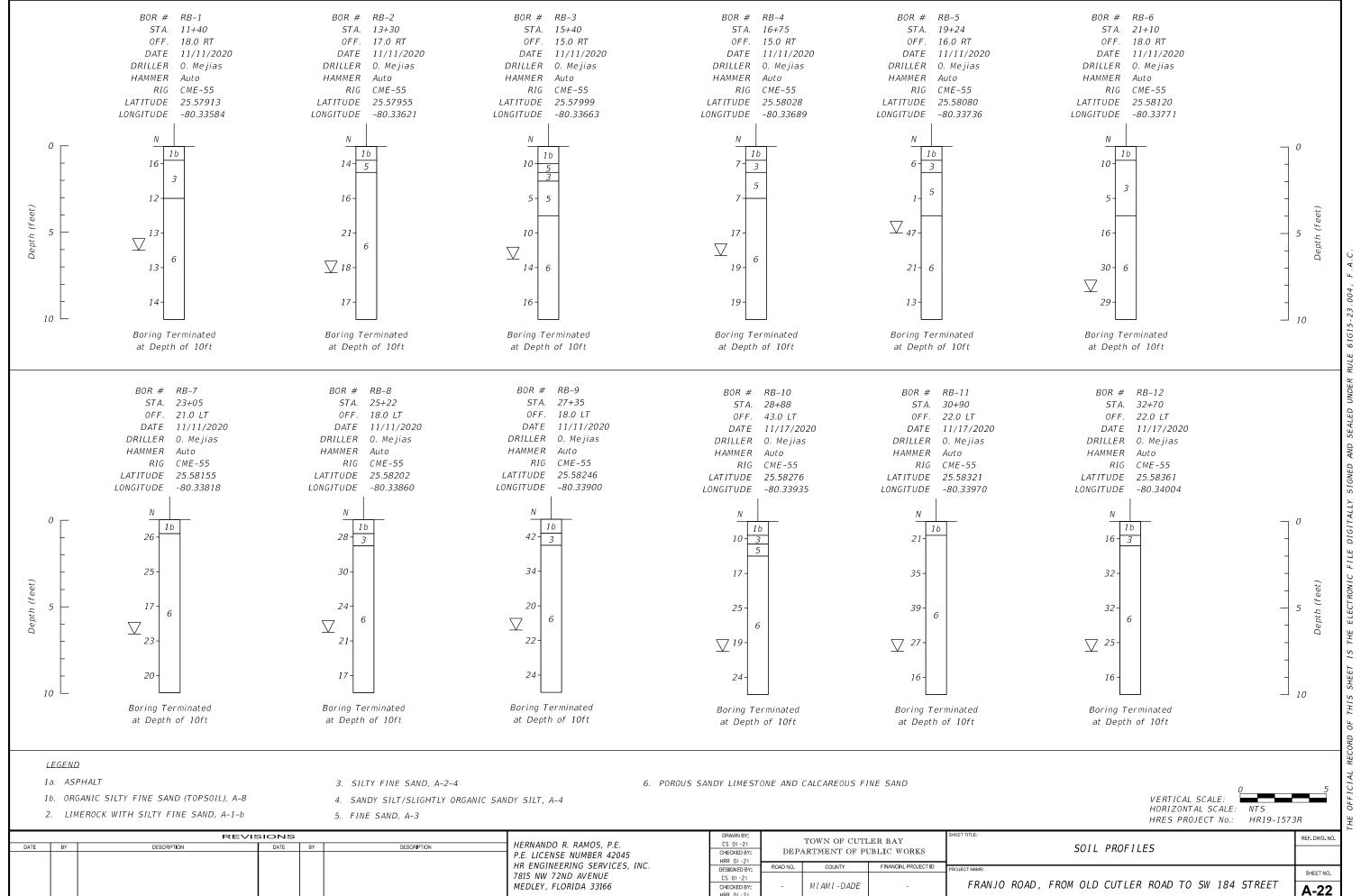
#### Notes:

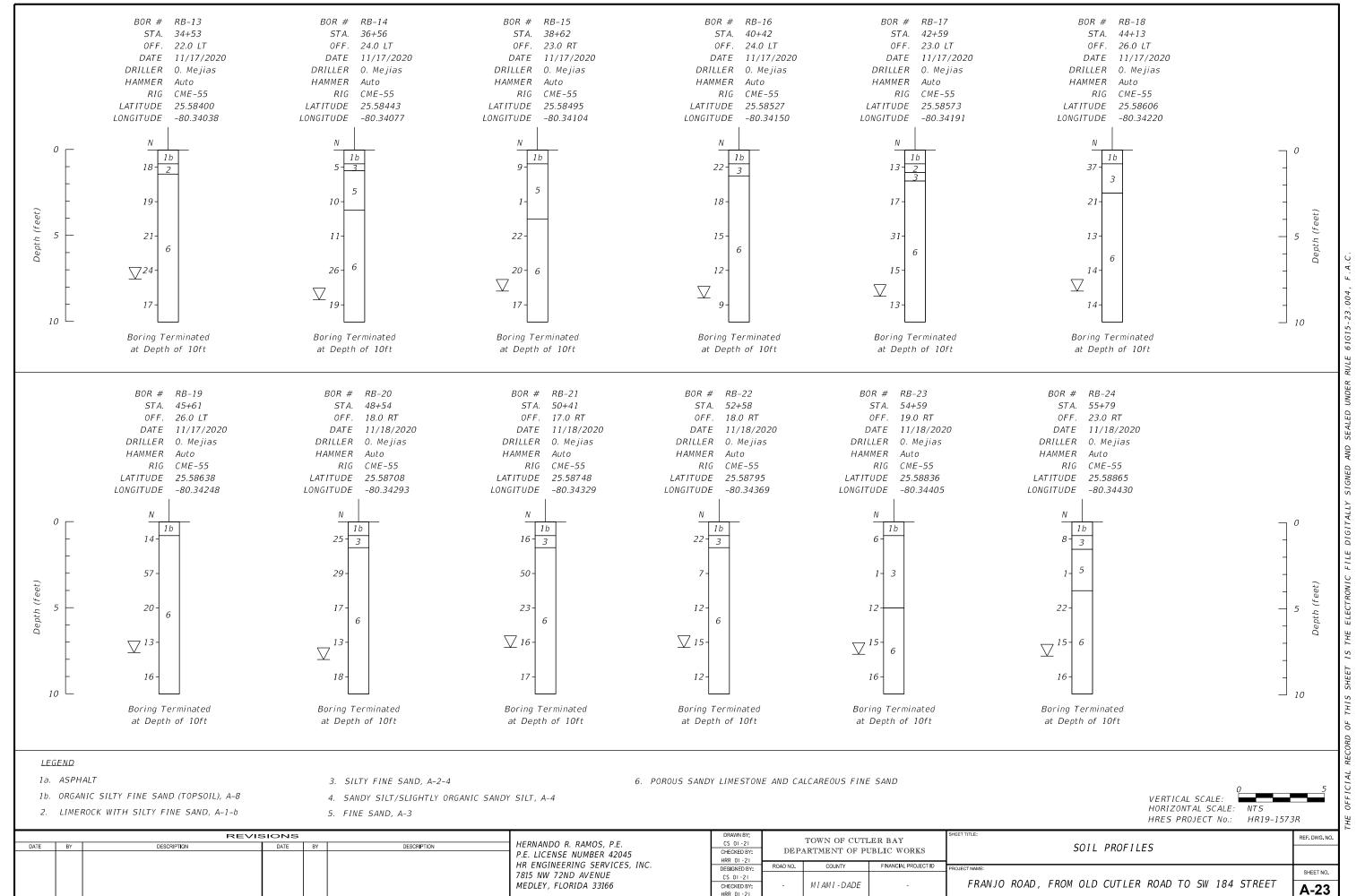
Plane coordinates were taken using a hand-held GPS and are approximate within 10 feet.

# FRANJO ROAD, FROM OLD CUTLER ROAD TO SW 184 STREET TOWN OF CUTLER BAY, DEPARTMENT OF PUBLIC WORKS SUMMARY OF PERCOLATION TEST RESULTS **USUAL-OPEN HOLE PERCOLATION RESULTS** HR ENGINEERING SERVICES, INC. HRES PROJECT NO. HR19-1573R **MIAMI-DADE COUNTY, FLORIDA JANUARY 29, 2021**

				<b>DEPTH TO</b>	DEPTH TO						
				WATER	WATER	HEAD,	HOLE	HOLE	RAT	RATE OF	k, HYDRAULIC
TEST	TEST	LATITUDE	LATITUDE LONGITUDE	BEFORE	DURING	Da	DEPTH,	DIAMETER,	FLO	FLOW, P	CONDUCTIVITY
No.	DATE			TEST, H	TEST,	نے	<u>#</u>	ס			cfs/ft^2-ft. Head
				ft.	ft.			inches	gpm	cfs	
P-1	12/01/20	25.58065	-80.33706	5.8	0.0	2.8	15.0	6.0	35.0	0.07799	7.1E-04
P-2	12/01/20	25.58248	-80.33888	7.5	0.0	7.5	15.0	6.0	33.2	0.07398	5.6E-04
P-3	12/01/20	25.58425	-80.34061	9.2	0.0	9.2	15.0	6.0	2.0	0.00446	3.0E-05
P-4	12/02/20	25.58607	-80.34219	10.8	0.0	10.8	15.0	6.0	1.0	0.00223	1.4E-05
P-5	12/02/20	25.58797	-80.34371	7.5	0.0	7.5	15.0	6.0	1.0	0.00223	1.7E-05
9-d	12/02/20	25.58998	-80.34531	5.7	0.0	2.5	15.0	6.0	34.0	0.07576	7.0E-04
P-7	12/02/20	25.59171	-80.34715	6.2	0.0	6.2	15.0	6.0	19.3	0.04300	3.7E-04
P-8	12/03/20	25.59390	-80.34804	4.8	0.0	4.8	15.0	6.0	34.9	0.07776	8.2E-04
P-9	12/03/20	25.59627	-80.34817	5.8	0.0	5.8	15.0	6.0	20.0	0.04456	4.0E-04

 $K_{15} = P/3.1416*d*Du\{Du/2+Ds\}$ , where Ds = Hole Depth - Hfor 0 to 15 ft.,

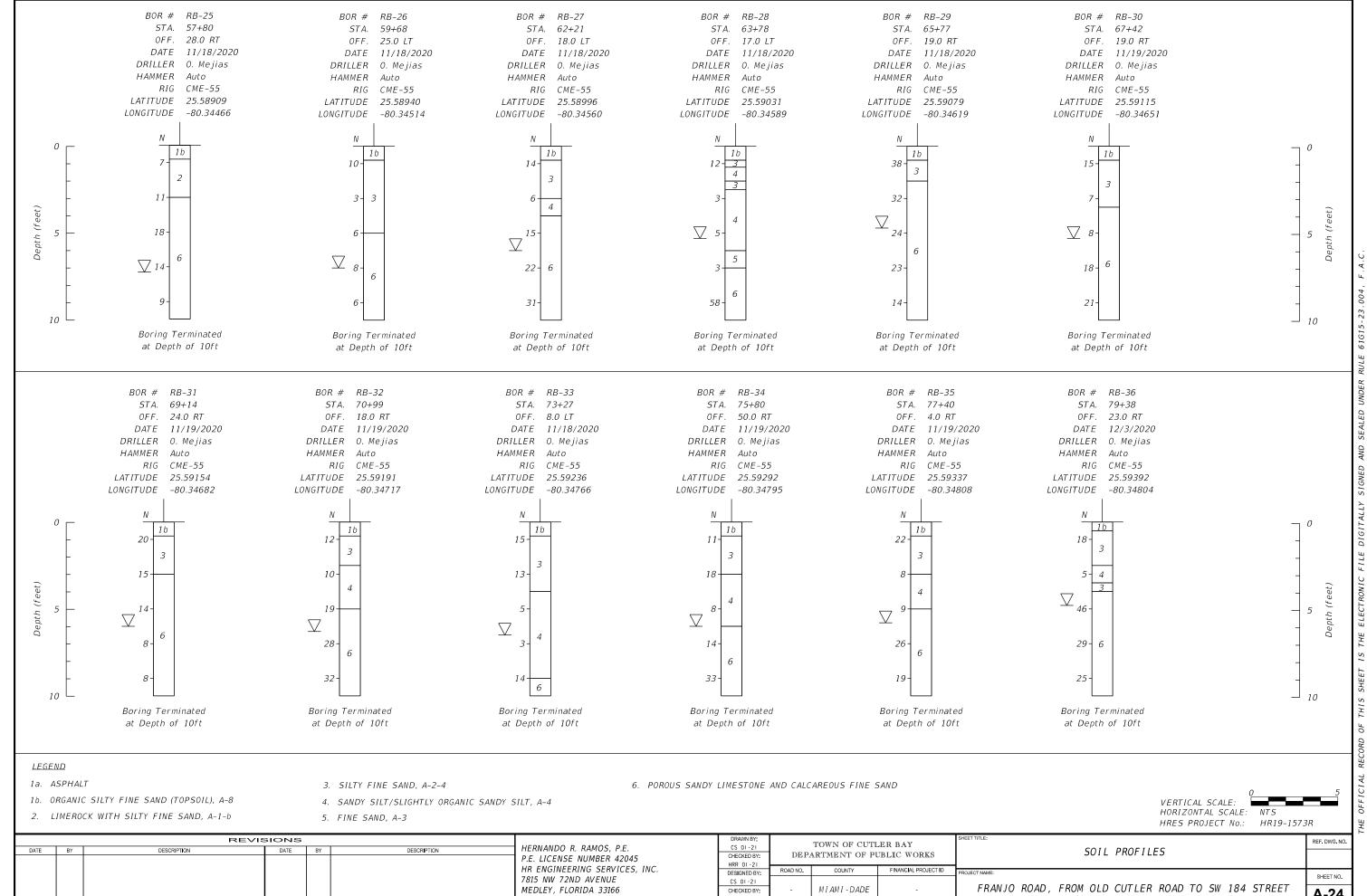




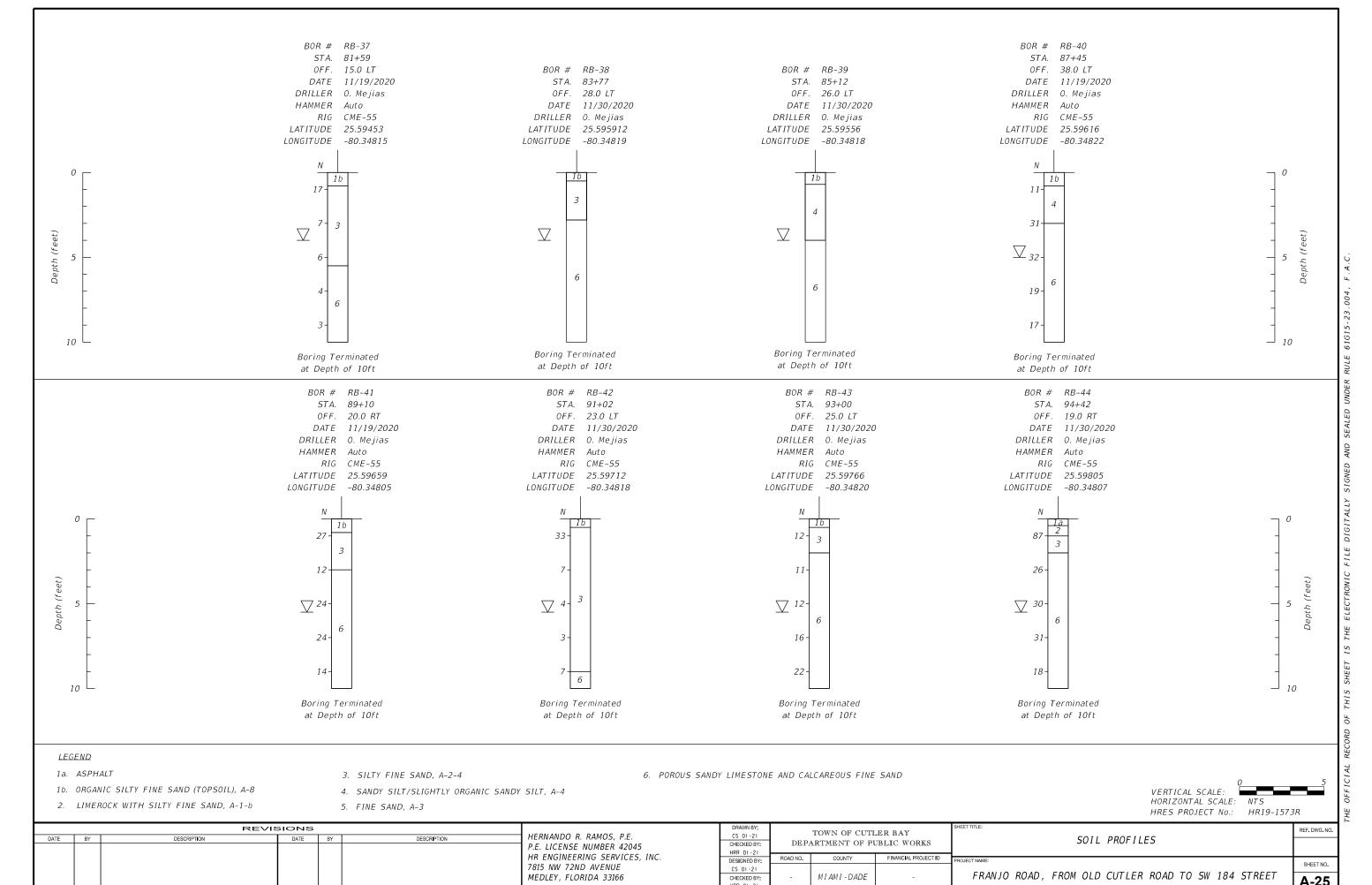
2

/2021 11·11·07 Δ

Y:\HR19-1573 Franjo Road, from Cutler Road to SW 184 Street - D6



A-24



2/4/2021 11:16:37 AM Y:\HR19-1573\_Franjo Road, from Cutler Road to SW 184 Street - D6 - RB

HRFSUse

TEST BORING No.	STATION	OFFSET ft.	RANGE IN DEPTH ft.	STRATUM	APPROXIMATE GROUNDWATER DEPTH ft.			
1101			0.0-0.8	1b	1			
RB-1	11+40	18.0 R	0.8-3.0	3	6.0			
			3.0-10.0	6	1			
			0.0-0.8	1b				
RB-2	13+30	17.0 R	0.8-1.5	5	7.3			
			1.5-10.0	6				
			0.0-1.0	1b				
			1.0-1.5	5				
RB-3	15+40	15.0 R	1.5-2.0	3	6.5			
			2.0-4.0	5				
			4.0-10.0	6				
			0.0-0.8	1b				
<b>DD</b> 4	10.75	45.00	0.8-1.5	3	1			
RB-4	16+75	15.0 R	1.5-3.0	5	6.3			
			3.0-10.0	6	1			
			0.0-0.5	1b				
P-1	18+17	58.0 R	0.5-3.0	3	5.7			
			3.0-15.0	6				
			0.0-0.8	1b				
DD 5	40.04	40 0 D	0.8-1.5	3	5.0			
RB-5	19+24	16.0 R	1.5-4.0	5	5.0			
			4.0-10.0	6	1			
			0.0-0.8	1b				
RB-6	21+10	18.0 R	0.8-4.0	3	8.4			
			4.0-10.0	6	1			
DD 7	00.05	24.0.1	0.0-0.8	1b	0.0			
RB-7	23+05	21.0 L	0.8-10.0	6	6.6			
			0.0-0.8	1b				
RB-8	25+22	18.0 L	0.8-1.5	3	6.5			
			1.5-10.0	6				
			0.0-0.5					
P-2	27+15	18.0 L	0.5-1.5	7.5				
			1.5-15.0	6	1			

TEST BORING	STATION	OFFSET ft.	RANGE IN DEPTH	STRATUM	APPROXIMATE GROUNDWATER DEPTH
No.			ft.		ft.
			0.0-0.8	1b	
RB-9	27+35	18.0 L	0.8-1.5	3	6.4
			1.5-10.0	6	
			0.0-0.8	1b	
RB-10	28+88	43.0 L	0.8-1.3	3	7.5
IND TO	20.00	40.0 L	1.3-2.0	5	] '
			2.0-10.0	6	
RB-11	30+90	22.0 L	0.0-1.0	1b	7.5
IND-11	30.30	22.0 L	1.0-1.5	6	7.0
			0.0-0.8	1b	
RB-12	32+70	22.0 L	0.8-1.4	3	7.5
			1.4-10.0	6	
			0.0-0.8	1b	
RB-13	34+53	22.0 L	0.8-1.4	2	7.5
			1.4-10.0	6	
			0.0-0.5	1b	
P-3	35+71	23.0 L	0.5-2.5 3		7.5
			2.5-15.0	6	
			0.0-0.8	1b	
RB-14	36+56	24.0 L	0.5-1.2	3	8.7
KD-14	30+30	24.0 L	1.2-3.5	5	0.7
			3.5-10.0	6	
			0.0-0.8	1b	
RB-15	38+62	23.0 R	0.8-4.0	5	8.2
			4.0-10.0	6	
			0.0-0.8	1b	
RB-16	40+42	24.0 L	0.8-1.5	3	8.6
			1.5-10.0	6	
			0.0-0.8	1b	
DD 47	40 : 50	23.0 L	0.8-1.3	0.5	
RD-1/	RB-17 42+59	∠3.0 L	1.3-1.8	3	8.5
			1.8-10.0	6	
			0.0-0.8	1b	
RB-18	44+13	26.0 L	0.8-2.5	3	8.2
			2.5-10.0	6	

TEST BORING No.	STATION	OFFSET ft.	RANGE IN DEPTH ft.	STRATUM	APPROXIMATE GROUNDWATER DEPTH ft.				
			0.0-0.5	1b					
P-4	44+16	26.0 L	0.5-2.5	3	10.8				
			2.5-15.0	6	1				
DD 10	45+61	26.0 L	0.0-0.8	1b	7.6				
RB-19	45+01	20.0 L	0.8-10.0	6	7.0				
			0.0-0.8	1b					
RB-20	48+54	18.0 R	0.8-1.5	3	8.0				
			1.5-10.0	6					
			0.0-0.8	1b					
RB-21	50+41	17.0 R	0.8-1.5	3	7.3				
			1.5-10.0	6					
			0.0-0.8 1b						
RB-22 52+58	18.0 R	0.8-1.5	3	7.3					
		1.5-10.0	6						
			0.0-0.5	1b					
D.E	E0.160	40 O D	0.5-4.0	3	7.5				
P-5	52+68	18.0 R	4.0-6.0	5	7.5				
			6.0-15.0	6					
			0.0-0.8	1b					
DD 00	54.50	40.0 D	0.8-2.0	3	7.7				
RB-23	54+59	19.0 R	2.0-5.0	3	7.7				
			5.0-10.0	6					
			0.0-0.8	1b					
DD 04	FF : 70	23.0 R	0.8-1.6	3	7.0				
KB-24	RB-24 55+79		1.6-4.0	5	7.8				
			4.0-10.0	6					
			0.0-0.8	1b					
RB-25	RB-25 57+80	28.0 R	0.8-3.0	2	7.3				
			3.0-10.0	6					
			0.0-0.8	1b					
RB-26	59+68	25.0 L	0.8-5.0	3	7.0				
			5.0-10.0	6					

TEST BORING	STATION	OFFSET ft.	RANGE IN DEPTH	STRATUM	APPROXIMATE GROUNDWATER DEPTH				
No.		<u> </u>	ft.	41.	ft.				
D.C.	64.65	64.0.0	0.0-0.5	1b					
P-6	61+65	64.0 R	0.5-5.5	3	5.7				
			5.5-15.0	6					
			0.0-0.8	1b					
RB-27	65+21	18.0 L	0.8-3.0	3 4	6.0				
			3.0-4.0	6	_				
			4.0-10.0						
			0.0-0.8	1b	-				
			0.8-1.2	3	_				
DD 00	62.70	17.0 L	1.2-2.0	4	5.3				
RB-28	63+78	17.0 L	2.0-2.5	3	5.3				
		2.5-6.0	<u>4</u> 5	4					
			6.0-7.0	-					
			7.0-10.0 6						
DD 00	05.77	40.0 D	0.0-0.8 1b		4.7				
RB-29	65+77	19.0 R	0.8-2.0	3	4.7				
			2.0-10.0 6						
RB-30	67+42	19.0 R	0.0-0.8	1b	5.3				
KD-30	07+42	19.0 K	0.8-3.5 3.5-10.0	6	5.5				
			0.0-0.8	1b					
RB-31	69+14	24.0 R	0.8-3.0	3	6.0				
170-31	09+14	24.01	3.0-10.0	6	0.0				
			0.0-0.5	1b					
P-7	70+34	23.0 L	0.5-3.5	4	6.2				
F-1	70+34	23.0 L	3.5-15.0	6	0.2				
				1b					
			0.0-0.8		+				
RB-32	70+99	18.0 R	0.8-2.5	3	6.3				
			2.5-5.0 5.0-10.0	6	-				
			0.0-0.8	1b	-				
RB-33	73+27	8.0 L	0.8-4.0	3 4	6.5				
			4.0-9.0		-				
		<u> </u>	9.0-10.0	6	<u> </u>				

TEST BORING No.	STATION	OFFSET ft.	RANGE IN DEPTH ft.	STRATUM	APPROXIMATE GROUNDWATER DEPTH ft.			
			0.0-0.8	1b				
			0.8-3.0	3	-			
RB-34	75+80	50.0 R	3.0-6.0	4	6.0			
			6.0-10.0	6	-			
			0.0-0.8	1b				
DD 05	40	405	0.8-3.0	3				
RB-35	77+40	4.0 R	3.0-5.0	4	5.8			
			5.0-10.0	6	1			
			0.0-0.5	1b				
			0.5-2.5	3	1			
P-8	79+30	23.0 R	2.5-3.5	4	4.8			
			3.5-4.0	3	1			
			4.0-15.0	6	1			
			0.0-0.5	1b				
			0.5-2.5	3	1			
RB-36	79+38	23.0 R	2.5-3.5	4	4.8			
			3.5-4.0	3	1			
			4.0-10.0	6				
			0.0-0.8	1b				
RB-37	81+59	15.0 L	0.8-5.5	3	4.0			
			5.5-10.0	6				
			0.0-0.5	1b				
RB-38	83+77	28.0 L	0.5-2.8	3	4.0			
			2.8-10.0	6				
			0.0-0.7	1b				
RB-39	85+12	26.0 L	0.7-4.0	4	4.0			
			4.0-10.0	6				
			0.0-0.8	1b				
RB-40	87+45	38.0 L	0.8-3.0	4	5.0			
			3.0-10.0	6				
			0.0-0.7	1b				
P-9	87+95	20.0 L	0.7-2.5	3	5.8			
1 -3	01.190	20.0 L	2.5-3.7	4	J.0			
			3.7-15.0	6				

DEPARTMENT OF PUBLIC WORKS MIAMI-DADE COUNTY, FLORIDA HR ENGINEERING SERVICES, INC. HRES PROJECT No. HR19-1573R JANUARY 29, 2021

TEST BORING No.	STATION	OFFSET ft.	RANGE IN DEPTH ft.	STRATUM	APPROXIMATE GROUNDWATER DEPTH ft.
140.			0.0-0.8	1b	14.
RB-41	89+10	20.0 R	0.8-3.0	3	5.5
			3.0-10.0	6	
			0.0-0.5	1b	
RB-42	91+02	23.0 L	0.5-9.0	3	5.5
			9.0-10.0	6	
			0.0-0.5	1b	
RB-43	93+00	25.0 L	0.5-2.0	3	5.5
			2.0-10.0	6	
			0.0-0.4	1a	
RB-44	94+42	19.0 R	0.4-1.0	2	5.5
110-44	<del>34</del> 742	19.0 K	1.0-2.0	3	5.5
			2.0-10.0	6	

#### **SOILS INFORMATION LEGEND**

STRATUM: 1a Asphalt

STRATUM: 1b Organic silty fine sand (topsoil), A-8
STRATUM: 2 Limerock with silty fine sand, A-1-b

**STRATUM: 3** Silty fine Sand, A-2-4

**STRATUM: 4** Sandy Silt/ Slightly organic sandy Silt, A-4

**STRATUM: 5** Fine Sand, A-3

**STRATUM: 6** Porous sandy Limestone and calcareous fine sand

#### FIELD TESTING PROCEDURES

<u>Test Borings</u> - The test borings were made in general accordance with ASTM-D-1586, "Penetration Test and Split-Barrel Sampling of Soils." The borings were advanced using a 3-inch ID casing and a rotary drilling process. At regular intervals, the drilling tools were removed and soil samples were obtained with a standard 1.4-inch I.D., 2-inch O.D., split-tube sampler. The sampler was first seated six inches and then driven an additional foot with blows of a 140-lb hammer falling 30 inches. The number of hammer blows required to drive the sampler the final foot is designated the "Penetration Resistance". The penetration resistance, when properly interpreted, is an index to the soil strength and density.

Representative portions of the soil samples, obtained from the sampler, were placed in glass jars and transported to our laboratory. An engineer then examined the samples in order to confirm the field classifications.

<u>Auger Borings</u> – Auger borings were mechanically advanced. The soils encountered were identified in the field from cuttings brought to the surface by the augering process.

Percolation Testing - The percolation tests were performed in order to estimate the hydraulic conductivity of the materials encountered. The usual open-hole Constant Head method was used. The general procedures outlined in the South Florida Water Management District (SFWD) were followed. Each test was performed in a 6.0-inch outside diameter hole initially pre-drilled to a depth of 15 feet below the existing ground surface, using an hollow stem auger. Each borehole was then filled with water and the water level maintained as close as possible to the ground surface. Once the inflow stabilized or came into equilibrium with the outflow rate or seepage, the amount of water added for a period of 10 minutes was recorded and the percolation rate calculated and reported in units of cfs/ft.²-ft. of head.

#### **APPENDIX B**

SUMMARY OF LABORATORY TEST RESULTS
ROADWAY SOILS SURVEY
LABORATORY TESTING PROCEDURES
LABORATORY TEST RESULTS
- SOIL TESTING

B-1 AND B-2 B-3 B-4

**B-5 THRU B-29** 

# FRANJO ROAD, FROM OLD CUTLER ROAD TO SW 184 STREET SUMMARY OF LABORATORY TEST RESULTS **TOWN OF CUTLER BAY**

nple, %	Fines	27	28	18	21	23	15	23	30	35	09	64	87	94	8
al in San	Sand	35	46	44	39	54	53	-	-	-	-	-	-	-	1
Materia	Gravel	38	26	38	40	23	32	-	-	-	-	-	-	-	1
Moisture Material in Sample, %	%	12	16	18	11	11	14	20	36	33	42	38	53	53	25
Organic Loss of	No. 100 No. 200 Ignition, %	-	-	1	-	-	-	-	-	ı	-	-	-	-	1
	No. 200	27	28	18	21	23	15	23	30	35	09	64	87	94	8
ing		31	32	22	25	27	19	-	-	-	-	-	-	-	-
rain Size Distribution - Percent Passing	No. 60	38	47	36	35	41	36	-	ı	-	-	-	-	-	ı
n - Perc	No. 40	43	56	45	44	53	20	-	-	-	-	-	-	-	-
tributio	No. 10	53	65	54	53	29	61	-	1	-	-	-	-	-	1
ize Dis	No. 4	62	74	62	09	77	89	ı	ı	-	-	1	-	-	1
Grain S	3/8"	75	06	92	71	92	22	-	-	1	-	-	-	-	1
	3/4"	93	100	100	77	100	91	ı	ı	-	1	-	-	1	ı
	1	100	100	100	100	100	100	-	-	-	-	-	-	-	-
Sample Depth	(ft)	0.8-1.5	0.8-1.5	0.8-1.4	0.8-1.3	0.8-1.5	0.8-2.0	2.0-3.0	3.0-4.0	4.0-5.0	3.0-4.0	1.2-2.0	2.5-4.0	4.0-6.0	0.7-0.9
Stratum	No.	3	3	2	2	3	2	8	3	3	4	4	4	4	2
AASHTO Stratum	Class.	A-2-4	A-2-4	A-1-b	A-1-b	A-2-4	A-1-b	A-2-4	A-2-4	A-2-4	A-4	A-4	A-4	A-4	A-3
Test Boring	No.	RB-1	RB-9	RB-13	RB-17	RB-20	RB-25	RB-26	RB-26	RB-26	RB-27	RB-28	RB-28	RB-28	RB-28

#### Fines Moisture Material in Sample, % 95 95 95 94 98 50 98 94 92 Sand Gravel Content 42 32 % 38 54 63 75 39 4 38 38 FRANJO ROAD, FROM OLD CUTLER ROAD TO SW 184 STREET No. 60 No. 100 No. 200 Ignition, % Loss of Organic 2 4 SUMMARY OF LABORATORY TEST RESULTS 92 94 86 95 98 94 95 92 50 **DEPARTMENT OF PUBLIC WORKS** HR ENGINEERING SERVICES, INC. MIAMI-DADE COUNTY, FLORIDA HRES PROJECT No. HR19-1573R **Grain Size Distribution - Percent Passing TOWN OF CUTLER BAY JANUARY 29, 2021** No. 40 No. 4 No. 10 3/8" 3/4" 2.5-4.0 4.0-5.0 4.0-6.0 3.0-4.0 4.0-6.0 6.0-8.0 8.0-9.0 3.0-4.0 0.8-2.0 4.0-5.0 Depth Œ AASHTO Stratum ģ 4 4 4 4 4 4 4 4 4 Class. A-4 Boring RB-35 RB-35 RB-40 RB-32 RB-33 RB-33 RB-33 RB-32 RB-34 RB-34 Š

NOVEMBER AND DECEMBER, 2020 DATE OF SURVEY: SURVEY MADE BY: HR Engineering Services, Inc. HERNANDO RAMOS. P.E.

SUBMITTED BY:

#### TOWN OF CUTLER BAY DEPARTMENT OF PUBLIC WORKS

DISTRICT:	
ROAD NO.:	
COUNTY:	MIAMI-DADE
COUNTY:	MIAMI-DADE

PROJECT NO.: --

PROJECT NAME:

FRANJO ROAD, FROM OLD CUTLER ROAD

TO SW 184TH STREET

#### CROSS SECTION SOIL SURVEY FOR THE DESIGN OF ROADS

SURVEY BEGINS STA.:

12+71.54 SURVEY ENDS STA. : 95+7.00

REFERENCE: FRANJO ROAD

-		ANIC TENT		STURE TENT				LYSIS RES PASS (%				ATTERBEF LIMITS (%					CORR	OSION	TEST RE	SULTS	
TRATUM J	NO. OF	%	NO. OF	MOISTURI	E_NO. OF	10	40	60	100	200	NO. OF	LIQUID	PLASTIC	AASHTO	DESCRIPTION	NO. OF	RESISTI	VITY_	CHLORIDE	SULFATES	рh
NO.	TESTS	ORGANIC	TESTS	CONTENT	TESTS	MESH	MESH	MESH	MESH	MESH	TESTS	LIMIT	INDEX	GROUP		TESTS	ohm-cn	n	ppm	ppm	
1a															ASPHALTIC CONCRETE						
1b														A-8	ORGANIC SILTY FINE SAND (TOPSOIL)						
2			3	18-11	3	61-53	50-44	36-35	25-19	21-15				A-1-b	LIMEROCK WITH SILTY FINE SAND (FILL)						
3			6	36-11	6	67-53	56-43	47-38	32-27	35-23				A-2-4	SILTY FINE SAND WITH TRACES OF LIMEROCK FRAGMENTS						
4	2	5-4	14	75-32	13					98-50				A-4	SANDY SILT/ SLIGHTLY ORGANIC SANDY SILT						
5			1	25	1					8				A-3	FINE SAND WITH TRACES OF LIMEROCK FRAGMENTS						
6															POROUS SANDY LIMESTONE AND CALCAREOUS FINE SAND						

#### EMBANKMENT AND SUBGRADE MATERIAL

STRATA BOUNDARIES ARE APPROXIMATE. MAKE FINAL CHECK AFTER GRADING.

GNE - GROUNDWATER NOT ENCOUNTERED

THE MATERIAL FROM STRATUM NUMBER IG IS ASPHALTIC CONCRETE.

THE MATERIAL FROM STRATUM NUMBER ID A-8 MATERIAL (TOPSOIL) AND IS UNSUITABLE FOR USE AS STABILIZED SUBGRADE OR FILL MATERIAL AND SHALL BE REMOVED.

THE MATERIAL FROM STRATUM NUMBER 2 IS A-I-b MATERIAL AND IS SUITABLE FOR USE AS GENERAL FILL WHEN UTILIZED IN ACCORDANCE WITH INDEX 120-001. IT CANNOT BE USED AS BASE MATERIAL.

THE MATERIAL FROM STRATUM NUMBER 3 IS A-2-4 MATERIAL AND IS SUITABLE FOR USE IN THE EMBANKMENT WHEN UTILIZED IN ACCORDANCE WITH INDEX 120-001. HOWEVER, THIS MATERIAL IS LIKELY TO RETAIN EXCESS MOISTURE AND BE DIFFICULT TO DRY AND COMPACT. IT SHALL BE USED IN THE EMBANKMENT ABOVE THE WATER LEVEL EXISTING AT THE TIME OF CONSTRUCTION.

THE MATERIAL FROM STRATUM NUMBER 4 IS A-4 MATERIAL, THIS MATERIAL IS UNSUITABLE FOR USE IN THE EMBANKMENT AND AS STABILIZED SUBGRADE AND SHALL BE REMOVED IN ACCORDANCE WITH STANDARD PLAN INDEX 120-002. IT SHALL BE REMOVED IF ENCOUNTERED WITHIN 2 FEET BELOW THE BOTTOM OF THE BASE. THIS MATERIAL WAS FOUND BY BORINGS RB-28, RB-30, RB-40 AND PERCOLATION TESTS P-7, P-8 AND P-9 AT DEPTHS WITHIN 2 FEET BELOW THE BOTTOM OF THE NEW BASE.

THE MATERIAL FROM STRATUM NUMBER 5 IS A-3 MATERIAL AND APPEARS SATISFACTORY FOR USE IN THE EMBANKMENT WHEN UTILIZED IN ACCORDANCE WITH INDEX 120-001.

THE MATERIAL FROM STRATUM NUMBER 6 IS THE NATURAL LIMESTONE. THIS MATERIAL TYPICALLY OFFERS A HIGH RESISTANCE TO EXCAVATION. SPECIAL EQUIPMENT AND BREAKING TOOLS MAY BE REQUIRED TO EXCAVATE IT. THIS MATERIAL IS ALSO DIFFICULT TO DEWATER DUE TO ITS HIGH POROSITY AND PERMEABILITY.

THE SYMBOL "--" REPRESENTS NO TESTING PERFORMED.

	REVIS	SIONS		
DATE	DESCRIPTION	DATE	DESCRIPTION	HERNANDO R. RAMOS, P.E.
				P.E. LICENSE NUMBER 42045
				HR ENGINEERING SERVICES, IN
				7815 NW 72ND AVENUE
				MEDLEY, FLORIDA 33166

	TOWN OF MIAMI L	AKES
ROAD NO.	COUNTY	PROJECT NO
	MIAMI-DADE	

ROADWAY SOILS SURVEY

SHEET NO.

B-3

#### LABORATORY TESTING PROCEDURES

<u>Organic Content (Organic Loss on Ignition)</u> – The amount of organic material in the sample was determined in this test, by measuring the loss due to ignition. The sample was first dried and weighed, then ignited and reweighed. The amount of organic material is expressed as a percentage of the soil weight. The test was conducted in general accordance with ASTM D-2974.

<u>Percent Fines Content</u> – In this test, the sample was dried and then washed over a # 200 mesh sieve. The percentage of soil by weight passing the sieve is the percentage of fines or portion of the sample in the silt and clay size range. This test was conducted in general accordance with ASTM D-1140.

<u>Moisture Content</u> – The moisture content (water content) is the ratio, expressed as a percentage of the weight of water in a given mass of soil to the weight of the soil particles. This test was conducted in general accordance with ASTM D-2216.

<u>Sieve Analysis</u> – The sieve analyses were performed to determine the particle size and distribution of sample tested. Each sample was dried, weighed, and washed over a # 200 mesh sieve. The dried sample was then passed through a standard set nested sieves to determine the grain size distribution of the soil particles coarser than the # 200 sieves. This test was conducted in general accordance with ASTM C-136.

## **GRAIN SIZE DATA SHEET**

#200

#40 #60 #100

### Size, mm.   Franjo Road   Project No.   HR19-1573R   100	GRAIN SIZE DISTRIBUTION CURVE 3/8" #4 #10 #40 #60 #100													/	1
tr Name:	GRAIN SIZE DISTI 1"3/4" 3/8" #4 #10									A		<b>/</b>			
g No.:         RB-1         Sample No.:         1B         Project No.:           re         Particle         Weight on Size, mm.         Accumulated Size, mm.         Percent Perce	73R			<u> </u>		80								issı	
g No.: RB-1 Sample No.: 1B  re Particle Weight on Accumulated Percent E Size, mm. Sieve, gr. Weight, gr. Retained 25.70 0.00 0.00 0  19.00 24.20 24.20 7  19.00 24.20 24.20 7  20.00 32.20 167.50 47  2.00 32.20 167.50 62  0.250 19.30 220.50 62  0.050 19.30 220.50 69  0.074 13.80 259.80 73	: HR19-157	- 1		REMAR			T		Classificat		A-2-4	ı			
g No.: RB-1 Sample No.: 11/18/2020 Sample No.: 11/18/2020 Accumulated E Size, mm. Sieve, gr. Weight, gr. 25.70 0.00 0.00 0.00 0.00 0.00 0.00 0.00	Project No.	_ Depth	Tested By	Percent	Passing	100	93	75	62	53	43	38	31	27	
g No.: RB-1  11/18/2020  11/18/2020  125.70  19.00  24.20  25.70  24.20  25.70  24.20  25.70  24.20  25.70  24.20  25.70  26.30  27.00  27.00  20.00  27.00		18		Percent	Retained	0	7	25	38	47	57	62	69	73	
g No.: RB-1  11/18/2020	anjo Road	Sample No.:		Accumulated	Weight, gr.	0.00	24.20	88.50	135.30	167.50	201.20	220.50	246.00	259.80	
g No.:	Ē	RB-1	1/18/2020	Weight on	Sieve, gr.	0.00	24.20	64.30	46.80	32.20	33.70	19.30	25.50	13.80	
Project N Boring N Date: Sieve Size 1 3/4" 44 40 60 60 200 PAN	ame:	.:.	-	Particle	Size, mm.	25.70	19.00	9.51	4.76	2.00	0.420	0.250	0.149	0.074	
	Project N	Boring N	Date:	Sieve	Size	-	3/4"	3/8"	4	10	40	09	100	200	PAN

0.01

		09		20		04	A		30 06	1	20	0				100 1 0.1	Grain Diameter, mm	
				bи	iss	34 }	uə:	orec	ł									
Classification:		A-2-4									7 or T 311) T 265)	(%)	38	19	16	27	12%	
62	53	43	38	31	27						S (AASHTO T 2 2216 (ASSHTO	Material in Sample (%)	≤ No. 4	>No. 4-≤ No. 40	>No. 40-≤ No. 200	>No. 200		
38	47	57	62	69	73			354.10	27%		ith ASTM C 136 with ASTM D 3	Mate	Gravel	Coarse Sand	Fine Sand	Silt and Clays	Water Content	
135.30	167.50	201.20	220.50	246.00	259.80			<b>-</b>	h Method=		ıl accordance w eral accordance							
46.80	32.20	33.70	19.30	25.50	13.80			Total Dry Weight Before Wash, (gr) =	Percent Finer than No. 200 Sieve by Wash Method=		Sieve Analysis Test performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311) Moisture Content Test performed in general accordance with ASTM D 2216 (ASSHTO T 265)			es, Inc.		}	\	
4.76	2.00	0.420	0.250	0.149	0.074			Weight Befor	iner than No.		llysis Test pe Content Test		Respectfully Submitted,	HR Engineering Services, Inc.	01			
4	10	40	09	100	200	PAN		Total Dry	Percent F		Sieve Ana Moisture		Respectful	HR Engin			D	

Hernando R. Ramos, P.E.

## **GRAIN SIZE DATA SHEET**

#200

#40 #60 #100

<b>I CURVE</b> #40 #60 #100	†															_	1
GRAIN SIZE DISTRIBUTION CURVE 3/8" #4 #10 #40 #50 #'	+		-	+	+	-				-			4				-
N 4	+									1	1						_
은				-													=
BU									1								_
ISTR #10									/								
DIS								/	7								
SIZE #4	F		-		-	_					-	-					_
S #							1										_
RAII	+=		1														_
ত																	
1" 3/4"	1			+	-	+					+	+				-	_
_	1																_
																	_
											+	=					_
	0		06		80		20	)		09		20		40	)		0
	100		σ		∞			•		9							
												bu	issi	₃d ł	.uə:	o e c	t
Γ.															1		
38			REMARKS				,	AASH I U									
.157	0.8-1.5	E.M.	IAR				È	AASH I U assificatio		A-2-4							
349	0.8	ш	REA				•	AAS		Þ							
ᄬ								္ဗီ									
.: 9	Depth:	By:	<u>.</u>	g													
Project No.: HR19-1573R	De	Tested By:	Percent	Passing	100	100	90	74	65	56	47	32	28				
roje		Tes	Per	Pas	-	_	١,				1						
Δ,															١,		_
			<u> </u>	þ												0	
	9		Percent	Retained	0	0	9	26	35	44	53	89	72			169.40	
	Ì		Pe	Ret												16	ı
																	L
	No.		ted	Jr.													
<u> </u>	Sample No.:		Accumulated	/eight, gr.	0.00	0.00	16.40	44.10	59.40	74.30	89.10	115.50	122.70				
Roa	am		cun	/eig	0	0	16	44	59	74	89	118	122				
Franjo Road	0)		Ac	3													
Fra																п	
			o	gr.			ا _			إ	إ					gr) =	
		20	ght	ve,	0.00	0.00	16.40	27.70	15.30	14.90	14.80	26.40	7.20			h,	
	RB-9	3/20	Weight on	Sieve, gr.		٦	Ť	2	-	-	-	7				Nas	
	교	11/18/2020														re	
		_	۵	Ė												3efo	
			Particle	Size, mm.	25.70	19.00	9.51	4.76	2.00	0.420	0.250	0.149	0.074			ht E	
ne:	ا		Par	Size	25	15	6	4	2	0.	0	o.	0.			Veig	
Project Name:	Boring No.:		ļ		$\vdash \vdash$											Total Dry Weight Before Wash, (gr) =	
ect	ing		Sieve	Size	_	3/4"	3/8"	4	10	40	09	100	200	PAN		J D	
Proj	Bor	Date:	Š	S		3	6		`	٧		7	2	۵		Tota	
<u></u>			-												4		
												Е	3-6				

Total Dry Weight Before Wash, (gr) =	169.40
Percent Finer than No. 200 Sieve by Wash Method=	28%
Sieve Analysis Test performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)	ith ASTM C 136 (AASHTO T 27 or T 31
Moisture Content Test performed in general accordance with ASTM D 2216 (ASSHTO T 265)	with ASTM D 2216 (ASSHTO T 265)

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16% 26 18 28 28 Material in Sample (%) Fine Sand >No. 40-≤ No. 200 Coarse Sand >No. 4-≤ No. 40 ≤ No. 4 >No. 200 Silt and Clays Water Content Gravel

HR Engineering Services, Inc.

Respectfully Submitted,

0.01

0.1

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100 0

Grain Diameter, mm

Hernando R. Ramos, P.E.

## **GRAIN SIZE DATA SHEET**

#200

#40 #60 #100

1ON CURVE #40 #60 #100	+												ß		<u> </u>	<b></b>	_
GRAIN SIZE DISTRIBUTION CURVE	7 7 7					~			A	/	<i>f</i>						
	100		06		80		7 02		;	09		03 Bu	issa	4 Ps		orec	, H
HR19-1573R	Depth: 0.8-1.4	E.M.	REMARKS				CH	Classification:		A-1-b							
Project No.: HR19-1573R	Depth:	Tested By:_	Percent	Passing	100	100	92	62	54	45	36	22	18				
	18		Percent	Retained	0	0	24	38	46	55	64	78	82			98.70	
Franjo Road	Sample No.:		Accumulated	Weight, gr.	0.00	0.00	23.30	37.80	45.50	53.90	62.80	77.40	81.30				
Fr	RB-13	11/18/2020	Weight on	Sieve, gr.	0.00	0.00	23.30	14.50	7.70	8.40	8.90	14.60	3.90			Total Dry Weight Before Wash, (gr) =	
ame:		11	Particle	Size, mm.	25.70	19.00	9.51	4.76	2.00	0.420	0.250	0.149	0.074			Weight Befor	
Project Name:	Boring No.:	Date:	Sieve	Size	1	3/4"	3/8"	4	10	40	09	100	200	PAN		Total Dry	
												E	3-7				

Total Dry Weight Before Wash, (gr) = Percent Finer than No. 200 Sieve by Wash Method=	98.70			
Sieve Analysis Test performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)  Moisture Content Test performed in general accordance with ASTM D 2216 (ASSHTO T 265)	with ASTM C 13 e with ASTM D	6 (AASHTO T 2 2216 (ASSHTO	27 or T 311) 7 T 265)	
	Mate	Material in Sample (%)	(%)	
Respectfully Submitted,	Gravel	≤ No. 4	38	
HR Engineering Services, Inc.	Coarse Sand	>No. 4-≤ No. 40	17	
	Fine Sand	Fine Sand >No. 40-≤ No. 200	27	
	Silt and Clays	>No. 200	18	
	Water Content		18%	

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Gravel ≤ No. 4  Coarse Sand >No. 4-≤ No. 40  Fine Sand >No. 40-≤ No. 200  Silt and Clays >No. 200	Material in Sample (%)
Coarse Sand No. 4-s No. 40 Fine Sand No. 40-s No. 200 Silt and Clays No. 200	38
Fine Sand No. 40-5 No. 200 Silt and Clays No. 200	17
	27
Water Content	18
Water Content	18%

0.01

0.1

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100

Grain Diameter, mm

Hernando R. Ramos, P.E.

## **GRAIN SIZE DATA SHEET**

#200

#40 #60 #100

TRIBUTION CURVE

GRAIN SIZE DIST HR19-1573R 11" 3/4" 3/8" #4 #10	100 0.8-1.3	E.M.	REMARKS 90		80		02	Classification:		A-1-b		20 Bu	isse	04 04	,erc
Project No.: HR19-1573R	Depth:_	Tested By:	Percent	Passing	100	77	71	09	53	44	35	25	21		
	18		Percent	Retained	0	23	29	40	47	56	65	75	79		229.00
Franjo Road	Sample No.:		Accumulated	Weight, gr.	00.0	52.90	67.30	92.50	107.20	127.40	148.60	172.50	180.00		
Fre	RB-17	1/27/2020	Weight on	Sieve, gr.	0.00	52.90	14.40	25.20	14.70	20.20	21.20	23.90	7.50		Total Dry Weight Before Wash, (gr) =
ame:		11	Particle	Size, mm.	25.70	19.00	9.51	4.76	2.00	0.420	0.250	0.149	0.074		Weight Befor
Project Name:	Boring No.:	Date:	Sieve	Size	1	3/4"	3/8"	4	10	40	09	100	200	PAN	Total Dry

Total Dry Weight Before Wash, (gr) =	229.00		
Percent Finer than No. 200 Sieve by Wash Method=	21%		
Sieve Analysis Test performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)	vith ASTM C 13	36 (AASHTO T	27 or T 311)
Moisture Content Test performed in general accordance with ASTM D 2216 (ASSHTO T 265)	e with ASTM D	2216 (ASSHTC	) T 265)
	Mate	Material in Sample (%)	(%)
Respectfully Submitted,	Gravel	≤ No. 4	40
HR Engineering Services, Inc.	Coarse Sand	Coarse Sand >No. 4-≤ No. 40	16
	Fine Sand	Fine Sand >No. 40-≤ No. 200	23
	Silt and Clays	>No. 200	21
	400400000000001		1000

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Mate	Material in Sample (%)	(%)
Gravel	≤ No. 4	40
oarse Sand	Coarse Sand >No. 4-≤ No. 40	16
Fine Sand	>No. 40-≤ No. 200	23
Silt and Clays	>No. 200	21
Water Content		11%

0.01

0.1

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100 0

Grain Diameter, mm

Hernando R. Ramos, P.E.

Florida Registration No. 42045

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## **GRAIN SIZE DATA SHEET**

#200

#40 #60 #100

GRAIN SIZE DISTRIBUTION CURVE 1"3/4" 3/8" #4 #10 #40 #60 #100	+ + + + + + + + + + + + + + + + + + + +					<i>A</i>	/	A	/	/	A			A			
	100		06		80		70	)	(	0.9		09 6u	issa	5 <b>q }</b>		oerc	30
HR19-1573R	0.8-1.5	E.M.	REMARKS				C I	Classification:		A-2-4							
Project No.: HR19-1573R	Depth:_	Tested By:	Percent	Passing	100	100	92	77	29	53	41	27	23				
	18		Percent	Retained	0	0	8	23	33	47	59	73	77			216.90	
Franjo Road	Sample No.:		Accumulated	Weight, gr.	0.00	0.00	17.20	49.10	70.80	101.20	126.90	157.60	167.80		•		
Fr	RB-20	12/3/2020	Weight on	Sieve, gr.	0.00	0.00	17.20	31.90	21.70	30.40	25.70	30.70	10.20			Total Dry Weight Before Wash, (gr) =	
ame:		+	Particle	Size, mm.	25.70	19.00	9.51	4.76	2.00	0.420	0.250	0.149	0.074		,	Weight Befor	
Project Name:	Boring No.:	Date:	Sieve	Size	-	3/4"	3/8"	4	10	40	09	100	200	PAN		Total Dry	
												E	3-9				

Total Dry Weight Before Wash, (gr) =	216.90	
Percent Finer than No. 200 Sieve by Wash Method=	23%	
Sieve Analysis Test performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311) Moisture Content Test performed in general accordance with ASTM D 2216 (ASSHTO T 265)	vith ASTM C 136 (A e with ASTM D 221	AASHTO T 27 or T 311) 6 (ASSHTO T 265)

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Mate	Material in Sample (%)	(%)
Gravel	≤ No. 4	23
Coarse Sand	>No. 4-≤ No. 40	24
Fine Sand	>No. 40-≤ No. 200	30
Silt and Clays	>No. 200	23
Water Content		11%

HR Engineering Services, Inc.

Respectfully Submitted,

Material in Sample (%)	. 4 23	No. 40 24	No. 200 30	200 23	11%	
rial in S	≤ No. 4	>No. 4-≤ No. 40	>No. 40-≤ No. 200	>No. 200		
Mate	Gravel	Coarse Sand	Fine Sand	Silt and Clays	Water Content	

0.01

0.1

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100 0

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Grain Diameter, mm

P.E.
Ramos, P.E.
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Hernando
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## **GRAIN SIZE DATA SHEET**

GRAIN SIZE DISTRIBUTION CURVE 1"3/4" 3/8" #4 #10 #40 #60 #100 #;	+ + + + + + + + + + + + + + + + + + + +							1	1	/	/					
	100		06		80		70		(	09		09 <b>6u</b>	isse	5 <b>9</b> },	o e ro	30
Project No.: HR19-1573R	0.8-2.0	E.M.	REMARKS				CHI	Classification:		A-1-b						
Project No.:	Depth:	Tested By:	Percent	Passing	100	91	77	89	61	50	36	19	15			
	<b>1</b> B		Percent	Retained	0	6	23	32	39	50	64	81	85		297.10	,
Franjo Road	Sample No.:		Accumulated	Weight, gr.	00.00	27.20	69.60	94.50	116.80	150.00	191.30	240.90	252.30			
. F	RB-25	12/3/2020	Weight on	Sieve, gr.	0.00	27.20	42.40	24.90	22.30	33.20	41.30	49.60	11.40		Total Dry Weight Before Wash, (gr) =	
ame:	.:0	4	Particle	Size, mm.	25.70	19.00	9.51	4.76	2.00	0.420	0.250	0.149	0.074		Weight Befo	
Project Name:	Boring No.:	Date:	Sieve	Size	-	3/4"	3/8"	4	10	40	09	100	200	PAN	Total Dry	
												В	-10	)		

Sieve Analysis Test performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311) Moisture Content Test performed in general accordance with ASTM D 2216 (ASSHTO T 265) 15% Percent Finer than No. 200 Sieve by Wash Method=

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Mate	Material in Sample (%)	(%)
Gravel	≤ No. 4	32
Coarse Sand	>No. 4-≤ No. 40	18
Fine Sand	>No. 40-≤ No. 200	35
Silt and Clays	>No. 200	15
Water Content		14%

HR Engineering Services, Inc.

Respectfully Submitted,

0.01

0.1

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100 0

Grain Diameter, mm

Hernando R. Ramos, P.E.

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name: F	ranjo Road		Project N	lo.: _	HR19-	-1573R		
Boring No.: RB-26	Sample No.:	2A	Dep	oth:	2.0	-3.0		
Date: 11/30/20								
Technician:			Į.	E.M				
Date Sample Placed in Oven:			11/3	30/202	20			
Time in / Out of Oven :		11/30/20	9:00 AM	то	12/01/20	9:00 AM		
Wt. of Wet Soil + Can, grams			24	47.10				
Wt. of Dry Soil + Can, grams			20	08.00				
Wt. of Can, grams No.	503	03 8.80						
Wt. of Dry Soil, grams			19	199.20				
Wt. of Moisture, grams		39.10						
Water Content, w%		20%						
Wt. of Dry Soil + Can Before Wash,	grams		20	08.00				
Wt. of Can, grams No.	503		8	3.80				
Wt. of Dry Soil Before Wash, grams			19	99.20				
Time in / Out of Oven :		12/02/20	9:30 AM	то	12/03/20	9:30 AM		
Wt. of Dry Soil + Can After Wash, g	rams		16	61.30				
Wt. of Dry Soil After Wash, grams			15	52.50				
Total Loss, grams			4	6.70				
Percent Finer Than No. 200 Sieve			2	23%				

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-2-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:	Fr	anjo Road		Project N	No.: _	HR19-	1573R	
Boring No.:	RB-26	Sample No.:	2B	Dep	oth: _	3.0	-4.0	
Date:						×		
Technician:					E.M			
Date Sample Pla	ced in Oven:			11/3	30/20	20		
Time in / Out of	Oven :		11/30/20	9:00 AM	то	12/01/20	9:00 AM	
Wt. of Wet Soil +	· Can, grams			14	49.40			
Wt. of Dry Soil +	Can, grams		112.00					
Wt. of Can, gram	ıs No.	504	8.80					
Wt. of Dry Soil, g	grams		103.20					
Wt. of Moisture,			37.40					
Water Content, v	n%		36%					
Wt. of Dry Soil +	Can Before Wash, ç	grams		1	12.00			
Wt. of Can, gram	ıs No.	504		:	8.80		E	
	Before Wash, grams			10	03.20	(		
Time in / Out of			12/02/20	9:30 AM	то	12/03/20	9:30 AM	
	· Can After Wash, gr	ams		8	31.50			
	After Wash, grams			7	2.70			
Total Loss, gran				3	30.50			
	nan No. 200 Sieve			;	30%			

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-2-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:	Fra	injo Road		Project	No.: _	HR19-	1573R	
Boring No.: RE	3-26	Sample No.:	3A	De	pth:	4.0	-5.0	
Date:11/30/2	20							
Technician:					E.M			
Date Sample Placed in	Oven:			11/	30/20	20		
Time in / Out of Oven :			11/30/20	9:00 AM	то	12/01/20	9:00 AM	
Wt. of Wet Soil + Can, g	grams			1	58.60			
Wt. of Dry Soil + Can, g	<b>Jrams</b>		121.20					
Wt. of Can, grams	No.	505	8.80					
Wt. of Dry Soil, grams			112.40					
Wt. of Moisture, grams			37.40					
Water Content, w%					33%			
Wt. of Dry Soil + Can B	efore Wash, gr	rams		1	21.20			
Wt. of Can, grams	No.	505			8.80			
Wt. of Dry Soil Before \	Wash, grams			1	12.40			
Time in / Out of Oven :			12/02/20	9:30 AM	то	12/03/20	9:30 AM	
Wt. of Dry Soil + Can A	.fter Wash, gra	ms			81.70			
Wt. of Dry Soil After Wa				•	72.90			
Total Loss, grams				;	39.50			
Percent Finer Than No.	. 200 Sieve				35%			

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265)
Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-2-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:	Fra	anjo Road		Project I	No.: _	HR19-	1573R		
Boring No.:	RB-27	Sample No.:	2B	De	pth:	3.0	-4.0		
Date:11	/30/20								
Technician:					E.M				
Date Sample Place	ed in Oven:			11/3	30/202	20			
Time in / Out of Ov	/en:		11/30/20	9:00 AM	то	12/01/20	9:00 AM		
Wt. of Wet Soil + C	Can, grams			1	47.60				
Wt. of Dry Soil + C	an, grams		106.70						
Wt. of Can, grams	No.	506	06 8.80						
Wt. of Dry Soil, gra	ams		97.90						
Wt. of Moisture, g	rams		40.90						
Water Content, w%	<b>%</b>				42%				
Wt. of Dry Soil + C	an Before Wash, g	rams		1	06.70				
Wt. of Can, grams	No.	506			8.80				
Wt. of Dry Soil Be	fore Wash, grams			Ç	97.90				
Time in / Out of Ov	ven:		12/02/20	9:30 AM	то	12/03/20	9:30 AM		
Wt. of Dry Soil + C	an After Wash, gra	ams		4	<b>17.90</b>				
Wt. of Dry Soil Aft					39.10				
Total Loss, grams					58.80				
Percent Finer Tha					60%				

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:	Franjo Road		Project No	.:HR19	-1573R			
Boring No.: RB-28	Sample No.:	1C	Deptl	n: <u>1.2</u>	-2.0			
Date: 11/30/20	_							
Technician:			E.	M				
Date Sample Placed in Oven:			11/30	/2020				
Time in / Out of Oven :		11/30/20	2:00 AM T	0 12/01/20	2:00 AM			
Wt. of Wet Soil + Can, grams			118	.90				
Wt. of Dry Soil + Can, grams		88.70						
Wt. of Can, grams No.	401	8.80						
Wt. of Dry Soil, grams		79.90						
Wt. of Moisture, grams			30.20					
Water Content, w%		8	38	%				
Wt. of Dry Soil + Can Before Was	sh, grams		88.	70				
Wt. of Can, grams No.	401		8.8	80				
Wt. of Dry Soil Before Wash, gra	ms		79.	.90				
Time in / Out of Oven :		12/02/20	1:00 PM T	O 12/03/20	1:00 PM			
Wt. of Dry Soil + Can After Wash	, grams		37.	.70				
Wt. of Dry Soil After Wash, gram			28.	.90				
Total Loss, grams			51	.00				
Percent Finer Than No. 200 Sieve	Э		64	%				

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:		Fra	injo Road		Project	No.:_	HR19-	1573R	
Boring No.:	RB-28		Sample No.:	2B	De	epth:	2.5	-4.0	
Date:	11/30/20								
Technician:						E.M		0	
Date Sample Pla	ced in Oven:				11	/30/20	20		
Time in / Out of	Oven :			11/30/20	2:00 AM	то	12/01/20	2:00 AM	
Wt. of Wet Soil +	· Can, grams					406.80			
Wt. of Dry Soil +	Can, grams			268.10					
Wt. of Can, gram	is l	No.	402			8.80		SS 200 200 200 200 200 200 200 200 200 2	
Wt. of Dry Soil, <u>զ</u>	grams			259.30					
Wt. of Moisture,	grams			138.70					
Water Content, v	<b>v</b> %			53%					
Wt. of Dry Soil +	Can Before	Wash, g	rams		268.10				
Wt. of Can, gran	ıs No.		402			8.80			
Wt. of Dry Soil E	Before Wash,	grams			i i	259.30	)		
Time in / Out of	Oven :			12/02/20	1:00 PM	то	12/03/20	1:00 PM	
Wt. of Dry Soil +	Can After W	ash, gra	ms			43.60			
Wt. of Dry Soil A	After Wash, g	rams				34.80			
Total Loss, gran	ns					224.50			
Percent Finer Th	nan No. 200 S	Sieve				87%			

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name: _	F	ranjo Road		Project N	No.: _	HR19-	1573R	
Boring No.:	RB-28	Sample No.:	3	Dep	oth:	4.0	-6.0	
Date:1	11/30/20							
Technician:					E.M			
Date Sample Plac	ced in Oven:			11/3	30/202	20		
Time in / Out of C	Oven :		11/30/20	2:00 AM	то	12/01/20	2:00 AM	
Wt. of Wet Soil +	Can, grams			5	17.70			
Wt. of Dry Soil +	Can, grams		341.10					
Wt. of Can, gram	s No.	403	9.00					
Wt. of Dry Soil, g	rams	v	332.10					
Wt. of Moisture,	grams		176.60					
Water Content, w	<b>/</b> %		53%					
Wt. of Dry Soil +	Can Before Wash,	grams		34	341.10			
Wt. of Can, gram	s No.	403			9.00			
Wt. of Dry Soil Bo	efore Wash, grams			3:	32.10			
Time in / Out of C	Oven :		12/02/20	1:00 PM	то	12/03/20	1:00 PM	
Wt. of Dry Soil +	Can After Wash, gr	ams		2	28.40			
Wt. of Dry Soil A	fter Wash, grams			1	9.40			
Total Loss, gram	s			3	12.70			
Percent Finer Th	an No. 200 Sieve				94%			

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name: Fra	anjo Road		Project l	No.: _	HR19-	1573R	
Boring No.: RB-28	Sample No.:	4A	De	pth:	6.0	-7.0	
Date:11/30/20							
Technician:				E.M			
Date Sample Placed in Oven:			11/3	30/20	20		
Time in / Out of Oven :		11/30/20	2:00 AM	то	12/01/20	2:00 AM	
Wt. of Wet Soil + Can, grams			4	50.30			
Wt. of Dry Soil + Can, grams		360.80					
Wt. of Can, grams No.	404	8.80					
Wt. of Dry Soil, grams			L				
Wt. of Moisture, grams		89.50					
Water Content, w%		25%					
Wt. of Dry Soil + Can Before Wash, g	rams		3	60.80			
Wt. of Can, grams No.	404			8.80			
Wt. of Dry Soil Before Wash, grams			3	52.00			
Time in / Out of Oven :		12/02/20	1:00 PM	то	12/03/20	1:00 PM	
Wt. of Dry Soil + Can After Wash, gra	ıms	334.40					
Wt. of Dry Soil After Wash, grams			3	25.60			
Total Loss, grams		AND AND THE PARTY OF THE PARTY		26.40			
Percent Finer Than No. 200 Sieve				8%			

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-3

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

-4.0
2:00 AM
4
1:00 PM

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name: _	Fr	anjo Road		Project	No.: _	HR19-	1573R
Boring No.:	RB-32	Sample No.:	3A	De	pth:_	4.0	-5.0
Date:1	1/30/20						
Technician:					E.M		
Date Sample Plac	ed in Oven:			11/	30/20	20	
Time in / Out of O	ven:		11/30/20	2:00 AM	то	12/01/20	2:00 AM
Wt. of Wet Soil +	Can, grams				343.50		
Wt. of Dry Soil + 0	Can, grams		251.10				
Wt. of Can, grams	s No.	408	9.00				
Wt. of Dry Soil, gr	ams		242.10				
Wt. of Moisture, g	rams		92.40				
Water Content, w	%				38%		
Wt. of Dry Soil + 0	Can Before Wash, (	grams		2	251.10		
Wt. of Can, grams	s No.	408			9.00		
Wt. of Dry Soil Be	fore Wash, grams			2	242.10		
Time in / Out of O	ven:		12/02/20	1:00 PM	то	12/03/20	1:00 PM
Wt. of Dry Soil + 0	Can After Wash, gr	ams			22.50		
Wt. of Dry Soil Af	ter Wash, grams		13.50				
Total Loss, grams	5				228.60		
Percent Finer Tha	an No. 200 Sieve				94%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted, AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:	F	ranjo Road		Project	No.: _	HR19-	1573R
Boring No.:	RB-33	Sample No.	3	De	pth:	4.0	-6.0
Date:	11/30/20						
Technician:					E.M		
Date Sample PI	aced in Oven:	1		11/	30/20	20	
Time in / Out of	Oven :		11/30/20	2:00 AM	то	12/01/20	2:00 AM
Wt. of Wet Soil	+ Can, grams			5	65.30		
Wt. of Dry Soil	+ Can, grams		370.50				
Wt. of Can, gra	ms No.	409	9.00				
Wt. of Dry Soil,	grams		361.50				
Wt. of Moisture	, grams		194.80				
Water Content,	<b>w</b> %				54%		
Wt. of Dry Soil	+ Can Before Wash,	grams		3	370.50	)	
Wt. of Can, gra	ms No.	409			9.00		
Wt. of Dry Soil	Before Wash, grams	<b>3</b>	361.50				
Time in / Out of	f Oven :		12/02/20	1:00 PM	то	12/03/20	1:00 PM
Wt. of Dry Soil	+ Can After Wash, g	rams			14.60		
Wt. of Dry Soil	After Wash, grams				5.60		
Total Loss, gra	ms				355.90	)	
Percent Finer 1	han No. 200 Sieve				98%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:	Fr	anjo Road		Project I	No.: _	HR19-	1573R
Boring No.: RB-3	3	Sample No.:	4	Dej	pth: _	6.0	-8.0
Date: 11/30/20							
Technician:					E.M		
Date Sample Placed in O	ven:		11/30/2020				
Time in / Out of Oven :			11/30/20	2:00 AM	то	12/01/20	2:00 AM
Wt. of Wet Soil + Can, gra	ams			4	80.10		
Wt. of Dry Soil + Can, gra	ams			2	97.20		
Wt. of Can, grams	No.	410	9.00				
Wt. of Dry Soil, grams			288.20				
Wt. of Moisture, grams				1	82.90		
Water Content, w%					63%		
Wt. of Dry Soil + Can Bef	ore Wash, g	ırams		2	97.20		
Wt. of Can, grams	No.	410			9.00		
Wt. of Dry Soil Before Wa	ash, grams			2	88.20		
Time in / Out of Oven :			12/02/20	1:00 PM	то	12/03/20	1:00 PM
Wt. of Dry Soil + Can Afte	er Wash, gra	ams		2	22.40		
Wt. of Dry Soil After Was					13.40		
Total Loss, grams				2	74.80		
Percent Finer Than No. 2	200 Sieve				95%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

Hernando R. Ramos, P.E.

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND ORGANIC CONTENT BY LOSS ON IGNITION

Project Name: Franjo Road		Project No	o.: HR′	19-1573R	
Boring No.: RB-33 Sample No.:	5A	Dept	:h:8	3.0-9.0	
Date:					
Technician:		E	.М		
Date Sample Placed in Oven:		11/30	0/2020		
Time in / Out of Oven :	11/30/20	2:00 AM T	O 12/01/20	2:00 AM	
Wt. of Wet Soil + Can, grams		40	1.90		
Wt. of Dry Soil + Can, grams		23:	3.70		
Wt. of Can, grams No. 507	7 8.90				
Wt. of Dry Soil, grams	224.80				
Wt. of Moisture, grams		16	8.20		
Water Content, w%		7:	5%		
Date Sample Placed in Furnace:		12/0	02/20		
Time in / out of furnace (minimum 6 hrs):	12/02/20	9:30 AM T	TO 12/02/20	3:30 PM	
Weight of Crucible & Oven-Dried Sample:		27	<b>7.40</b>		
Weight of Crucible and Sample After Ignition:	26.80				
Weight of Crucible: No. 227					
Weight of Oven-Dried Soil:	11.40				
Weight Loss due to Ignition:			.60		
Percent Organics:		CALLANDA AND CONTRACTOR OF THE PROPERTY OF THE PARTY OF T	5%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Organic Content Test was performed in general accordance with ASTM D 2974 (AASHTO T 267)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:	Fra	anjo Road		Project	No.: _	HR19-	1573R
Boring No.: RB-33	3	Sample No.:	5A	De	pth:	8.0	-9.0
Date: 11/30/20							
Technician:		P			E.M		
Date Sample Placed in Ov	en:		11/30/2020				
Time in / Out of Oven :			11/30/20	2:00 AM	то	12/01/20	2:00 AM
Wt. of Wet Soil + Can, gra	ms				101.90		
Wt. of Dry Soil + Can, grai	ms		=	2	233.70	<u> </u>	
Wt. of Can, grams	No.	507	8.90				
Wt. of Dry Soil, grams			224.80				
Wt. of Moisture, grams		1	168.20				
Water Content, w%					75%		
Wt. of Dry Soil + Can Befo	ore Wash, g	rams		2	222.10		
Wt. of Can, grams	No.	507			8.90		
Wt. of Dry Soil Before Wa	sh, grams		213.20				
Time in / Out of Oven :			12/02/20	1:00 PM	то	12/03/20	1:00 PM
Wt. of Dry Soil + Can Afte	r Wash, gra	ams		,	114.60	)	
Wt. of Dry Soil After Wash				,	105.70		
Total Loss, grams				,	107.50	)	
Percent Finer Than No. 20	00 Sieve				50%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name: Fra	anjo Road		Project	No.: _	HR19-	1573R
Boring No.: RB-34	Sample No.:	2B	De	pth:	3.0	-4.0
Date:11/30/20						
Technician:				E.M		
Date Sample Placed in Oven:		11/30/2020				
Time in / Out of Oven :	3	11/30/20	2:00 AM	то	12/01/20	2:00 AM
Wt. of Wet Soil + Can, grams			2	279.50		
Wt. of Dry Soil + Can, grams		202.90				
Wt. of Can, grams No.	508	8.90				
Wt. of Dry Soil, grams		194.00				
Wt. of Moisture, grams				76.60		
Water Content, w%				39%		
Wt. of Dry Soil + Can Before Wash, g	rams	1	2	202.90		
Wt. of Can, grams No.	508			8.90		
Wt. of Dry Soil Before Wash, grams		194.00				
Time in / Out of Oven :		12/02/20	1:00 PM	то	12/03/20	1:00 PM
Wt. of Dry Soil + Can After Wash, gra	ıms			13.70		
Wt. of Dry Soil After Wash, grams				4.80		
Total Loss, grams			1	189.20	)	
Percent Finer Than No. 200 Sieve				98%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name:F	ranjo Road		Project	No.:_	HR19-	1573R
Boring No.: RB-34	Sample No.:	3 Depth: 4.0-6.0				-6.0
Date:11/30/20						
Technician:				E.M		
Date Sample Placed in Oven:		11/30/2020				
Time in / Out of Oven :		11/30/20	2:00 PM	то	12/01/20	2:00 PM
Wt. of Wet Soil + Can, grams			8	67.60		
Wt. of Dry Soil + Can, grams		617.80				
Wt. of Can, grams No.	509	8.90				
Wt. of Dry Soil, grams		608.90				
Wt. of Moisture, grams		249.80				
Water Content, w%		41%				
Wt. of Dry Soil + Can Before Wash,	grams		6	17.80		
Wt. of Can, grams No.	509			8.90		
Wt. of Dry Soil Before Wash, grams			6	08.90		
Time in / Out of Oven :		12/02/20	1:00 PM	то	12/03/20	1:00 PM
Wt. of Dry Soil + Can After Wash, gr	ams		4	43.80		
Wt. of Dry Soil After Wash, grams			;	34.90		
Total Loss, grams			5	74.00		
Percent Finer Than No. 200 Sieve				94%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

Hernando R. Ramos, P.E.

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name: Fra	njo Road		Project N	No.: _	HR19-	1573R
Boring No.: RB-35	Sample No.:	2B	Dep	oth: _	3.0	-4.0
Date: 12/02/20						
Technician:				E.M		
Date Sample Placed in Oven:		12/02/2020				
Time in / Out of Oven :		12/02/20	10:30 AM	то	12/03/20	10:30 AM
Wt. of Wet Soil + Can, grams			1:	94.20		
Wt. of Dry Soil + Can, grams		143.10				
Wt. of Can, grams No.	510	8.80				
Wt. of Dry Soil, grams		134.30				
Wt. of Moisture, grams		51.10				
Water Content, w%		38%				
Wt. of Dry Soil + Can Before Wash, gr	rams		1-	43.10		
Wt. of Can, grams No.	510			8.80		
Wt. of Dry Soil Before Wash, grams		134.30				
Time in / Out of Oven :		12/02/20	1:00 PM	то	12/03/20	1:00 PM
Wt. of Dry Soil + Can After Wash, gra	ms		1	15.80		
Wt. of Dry Soil After Wash, grams		angen ten er en	PLOTE STATE OF THE PARTY OF THE	7.00		
Total Loss, grams				27.30		
Percent Finer Than No. 200 Sieve				95%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND PERCENT PASSING THE No. 200 SIEVE

Project Name: Fra	anjo Road		Project	No.: _	HR19-	1573R
Boring No.: RB-35	Sample No.:	3A	De	pth: _	4.0	-5.0
Date:12/02/20						
Technician:				E.M		
Date Sample Placed in Oven:		12/02/2020				
Time in / Out of Oven :		12/02/20	10:30 AM	то	12/03/20	10:30 AM
Wt. of Wet Soil + Can, grams			4	30.30		
Wt. of Dry Soil + Can, grams			3	15.30	1	
Wt. of Can, grams No.	611	8.80				
Wt. of Dry Soil, grams		306.50				
Wt. of Moisture, grams			1	15.00		
Water Content, w%				38%		
Wt. of Dry Soil + Can Before Wash, g	rams		3	315.30		
Wt. of Can, grams No.	611			8.80		
Wt. of Dry Soil Before Wash, grams		306.50				
Time in / Out of Oven :		12/02/20	1:00 PM	то	12/03/20	1:00 PM
Wt. of Dry Soil + Can After Wash, gra	ams			32.80		
Wt. of Dry Soil After Wash, grams				24.00		
Total Loss, grams	1			282.50	1	
Percent Finer Than No. 200 Sieve				92%		

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Fines Content Test was performed in general accordance with ASTM C 136 (AASHTO T 27 or T 311)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

A-4

7815 N.W. 72nd Avenue - Medley, Florida 33166 Phone (305) 888-8880, Fax (305) 888-8770

## REPORT OF MOISTURE AND ORGANIC CONTENT BY LOSS ON IGNITION

Wt. of Wet Soil + Can, grams       412.40         Wt. of Dry Soil + Can, grams       313.70         Wt. of Can, grams       8.90         Wt. of Dry Soil, grams       304.80         Wt. of Moisture, grams       98.70         Water Content, w%       32%	E.M 12/02/2020 12/02/20 10:30 AM TO 12/03/20 10:30 AM 412.40 313.70
Date: 12/02/20           Technician:         E.M           Date Sample Placed in Oven:         12/02/2020           Time in / Out of Oven:         12/02/20 10:30 AM TO 12/03/20 10:30 A           Wt. of Wet Soil + Can, grams         412.40           Wt. of Dry Soil + Can, grams         313.70           Wt. of Can, grams         8.90           Wt. of Dry Soil, grams         304.80           Wt. of Moisture, grams         98.70           Water Content, w%         32%	12/02/2020 12/02/20 10:30 AM TO 12/03/20 10:30 AM 412.40 313.70
Date Sample Placed in Oven:   12/02/2020	12/02/2020 12/02/20 10:30 AM TO 12/03/20 10:30 AM 412.40 313.70
Time in / Out of Oven :       12/02/20       10:30 AM TO 12/03/20       10:30 A         Wt. of Wet Soil + Can, grams       412.40         Wt. of Dry Soil + Can, grams       313.70         Wt. of Can, grams       8.90         Wt. of Dry Soil, grams       304.80         Wt. of Moisture, grams       98.70         Water Content, w%       32%	12/02/20 10:30 AM TO 12/03/20 10:30 AM 412.40 313.70
Wt. of Wet Soil + Can, grams       412.40         Wt. of Dry Soil + Can, grams       313.70         Wt. of Can, grams       8.90         Wt. of Dry Soil, grams       304.80         Wt. of Moisture, grams       98.70         Water Content, w%       32%	412.40 313.70
Wt. of Dry Soil + Can, grams       313.70         Wt. of Can, grams       8.90         Wt. of Dry Soil, grams       304.80         Wt. of Moisture, grams       98.70         Water Content, w%       32%	313.70
Wt. of Can, grams         No.         612         8.90           Wt. of Dry Soil, grams         304.80           Wt. of Moisture, grams         98.70           Water Content, w%         32%	
Wt. of Dry Soil, grams  Wt. of Moisture, grams  98.70  Water Content, w%  304.80	
Wt. of Moisture, grams  98.70  Water Content, w%  32%	612 8.90
Wt. of Moisture, grams 98.70 Water Content, w% 32%	304.80
Water Content, w% 32%	98.70
Date Sample Placed in Eurnace: 12/03/20	32%
Date dample i laced in i difface.	12/03/20
	s): 12/03/20 11:00 AM TO 12/03/20 5:00 PM
Weight of Crucible & Oven-Dried Sample: 26.20	le: 26.20
Weight of Crucible and Sample After Ignition: 25.80	
Weight of Crucible: No. 54 15.00	
Weight of Oven-Dried Soil: 11.20	11.20
Weight Loss due to Ignition: 0.40	
Percent Organics: 4%	0.40

Moisture Content Test was performed in general accordance with ASTM D 2216 (AASHTO T 265) Organic Content Test was performed in general accordance with ASTM D 2974 (AASHTO T 267)

Respectfully Submitted,

AASHTO Classification:

HR Engineering Services, Inc.

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#### **APPENDIX C**

**GTR REVIEW CHECKLIST** 

C-1 AND C-2

#### GTR REVIEW CHECKLIST FOR SITE INVESTIGATION

#### A. Site Investigation Information

Since the most important step in the geotechnical design process is to conduct an <u>adequate</u> site investigation, presentation of the subsurface information in the geotechnical report and on the plans deserves careful attention.

Geo	technical Report Text (Introduction) (Pgs. 10-1 to 10-4)	Yes	<u>No</u>	Unknown or N/A
1.	Is the general location of the investigation described and/or a vicinity map included?			
2.	Is scope and purpose of the investigation summarized?			
3.	Is concise description given of geologic setting and topography of area?			
4.	Are the field explorations and laboratory tests on which the report is based listed?			
5.	Is the general description of subsurface soil, rock, and groundwater conditions given?			
*6.	Is the following information included with the geotechnic report (typically included in the report appendices):	cal		
	a. Test hole logs? (Pgs. 2-24 to 2-32)			
	b. Field test data?			
	c. Laboratory test data? (Pgs. 4-22 to 4-23)			
	d. Photographs (if pertinent)?			
Plan	and Subsurface Profile (Pgs. 2-19, 3-9 to 3-12, 10-13)			
*7.	Is a plan and subsurface profile of the investigation site provided?			
8.	Are the field explorations located on the plan view?			

<sup>\*</sup>A response other than (yes) or (N/A) for any of these checklist questions is cause to contact the appropriate geotechnical engineer for a clarification and/or to discuss the project.

A. <u>Site</u>	Investigation Information (Cont.)	Yes	<u>No</u>	Unknown or N/A
*9.	Does the conducted site investigation meet minimum criteria outlined in Table 2?			
10.	Are the explorations plotted and correctly numbered on the profile at their true elevation and location?			
11.	Does the subsurface profile contain a word description and/or graphic depiction of soil and rock types?			
12.	Are groundwater levels and date measured shown on the subsurface profile?			
Subs	surface Profile or Field Boring Log (Pgs. 2-14, 2-15, 2-24	to 2-31)		
13.	Are sample types and depths recorded?			
*14.	Are SPT blow count, percent core recovery, and RQD values shown?			
15.	If cone penetration tests were made, are plots of cone resistance and friction ratio shown with depth?			
<u>Lab</u>	oratory Test Data (Pgs. 4-6, 4-22, 4-23)			
*16.	Were lab soil classification tests such as natural moisture content, gradation, Atterberg limits, performed on selected representative samples to verify field visual soil identification?			
17.	Are laboratory test results such as shear strength (Pg. 4-14), consolidation (Pg. 4-9), etc., included and/or summarized?			

<sup>\*</sup>A response other than (yes) or (N/A) for any of these checklist questions is cause to contact the appropriate geotechnical engineer for a clarification and/or to discuss the project.